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SEISMIC VULNERABILITY OF MULTI-LEAF, HERITAGE MASONRY WALLS USING ELASTO-PLASTIC DAMAGE MODEL



By
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OUTLINE



1. Introduction.
2. Subject and Scope
3. Experimental Study at ITU
4. FEM Simulations
5. Conclusions
6. Work in Progress at KFUPM





Introduction



- Masonry structures are also commonly used in Saudi Arabia for the construction of low rise buildings especially in small towns and villages due to economy in the cost of construction.
- There exists a rich heritage of URM structures in the Western region of Saudi Arabia.
- With only a few incidences of major earthquakes in the Kingdom in the recent history, research into seismic retrofitting of masonry structures is rather scant.



Aim and Scope



In this study

- An experimental study on 'multi-leaf stone masonry walls', carried out in ITU Structural and EQ Engineering Laboratory is briefly introduced.
- Finite element analysis results of two experimental studies (Demir et al. (2011) and Nanni et al. (2005) are presented.
- Axial stress vs. Shear strength curves for both studies are obtained via FEA and compared.
- Ongoing efforts in KFPUM are indicated.



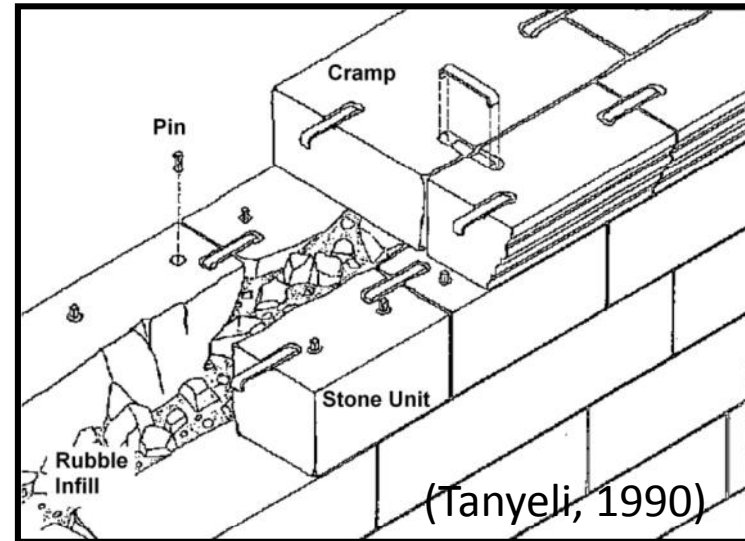
**EXPERIMENTAL STUDY
CARRIED OUT IN ITU (Demir
et al., 2011)**



INTRODUCTION TO THE EXPERIMENTAL PROGRAM



- The structure of the walls in monumental buildings constructed in the classical period of the Ottoman Empire usually consist of:
 - The two-layer walls of stone, often cut in the outer wall.
 - Inner layer of rubble filling
 - Iron clamp (and in some cases, shear pins) connected to each other block.
- The reinforcement of stone anchors units is molten lead.

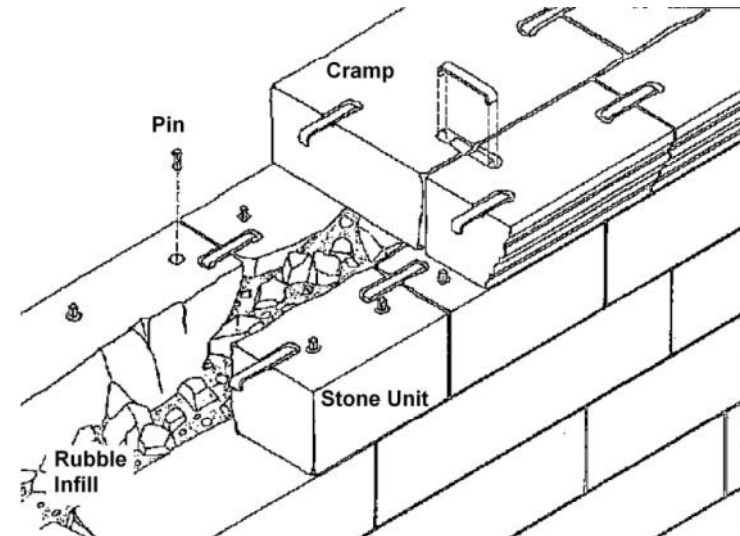




INTRODUCTION TO THE EXPERIMENTAL PROGRAM



Bayezid Mosque-15. century (Cut Stone Wall)

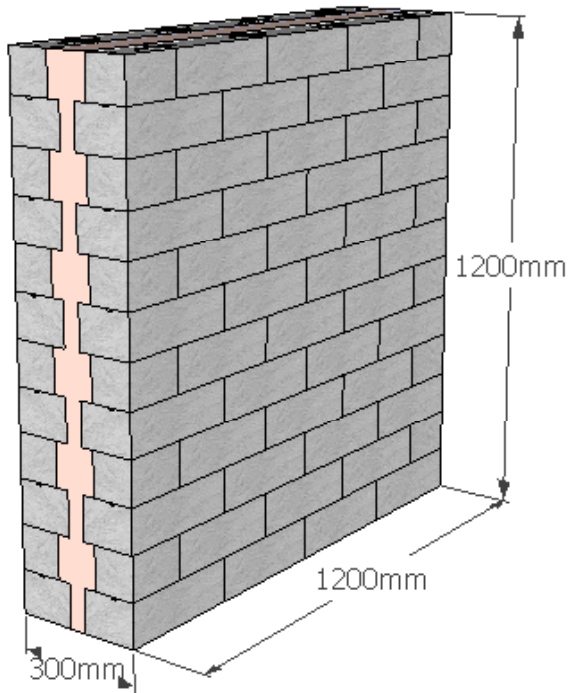




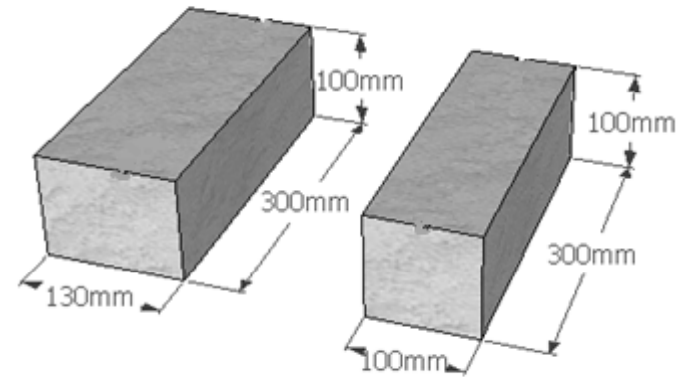
EXPERIMENTAL PROGRAM



1 / 3 scale wall specimens



Wall Model



Stone blocks



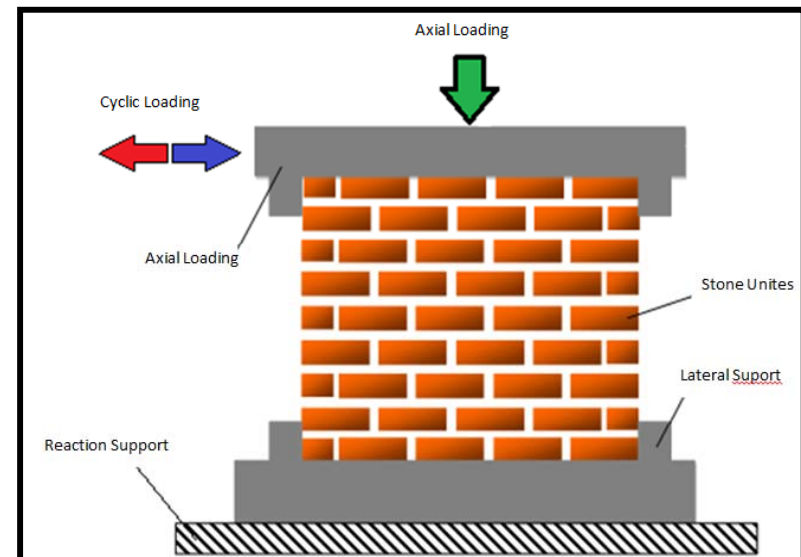
EXPERIMENTAL PROGRAM



Wall Tests

- The tests were done at ITU Structural and EQ Engineering Laboratory.
- The variables are
 - normal stress level,
 - and clamps
- For each axial load, horizontal cyclic force is applied in the in-plane direction

sample	clamp	Axial Load (MPa)	variable
M-25-C	Yes	0.25	axial stress
M-50-C	Yes	0.50	axial stress
M-75-C	Yes	0.75	axial stress
M-100-C	Yes	1.00	axial stress
M-50	No	0.50	Clamp not used

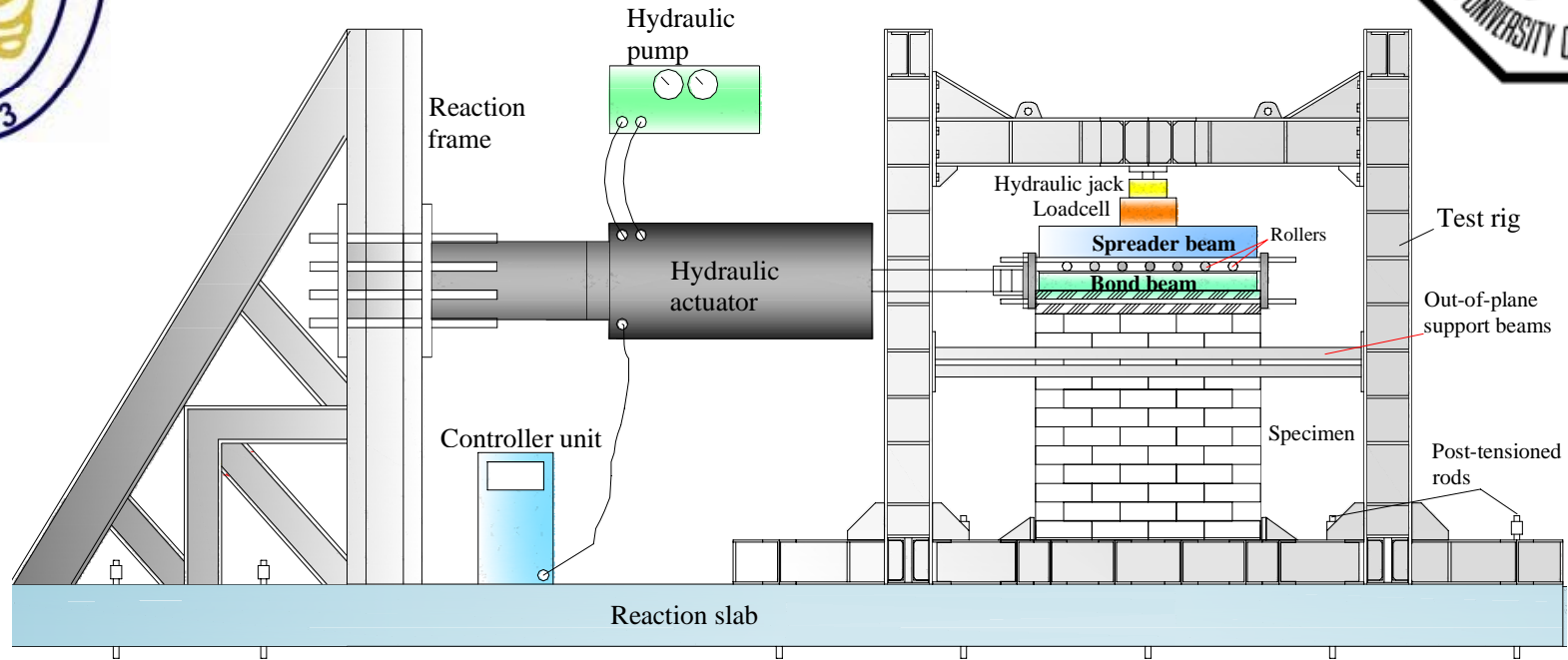




EXPERIMENTAL PROGRAM



Wall Tests





EXPERIMENTAL PROGRAM

Wall Tests





FEM Simulations



FEM Simulations



- FEM analysis has been used to model and simulate the behavior of masonry wall subjected to in-plane loading
- Concrete-damage plasticity (CDP) model available in ABAQUS has been adopted for this purpose. This model provides a good prediction for the behavior of such structures with cyclic loading
- ABAQUS software has been used for the simulation. ABAQUS provides an extraordinary capability to simulate this type of structure under cyclic loading using the desired model.



Elasto-Plastic Damage Model



The main purpose of the study is to find the interaction diagram between the normal stress and lateral shear strength of the wall.

For this purpose, a Finite element simulation has been carried out for two walls:

- The wall tested at ITU by Demir et al. (2011)
- A hollow block concrete wall studied by Nanni et al (2005)

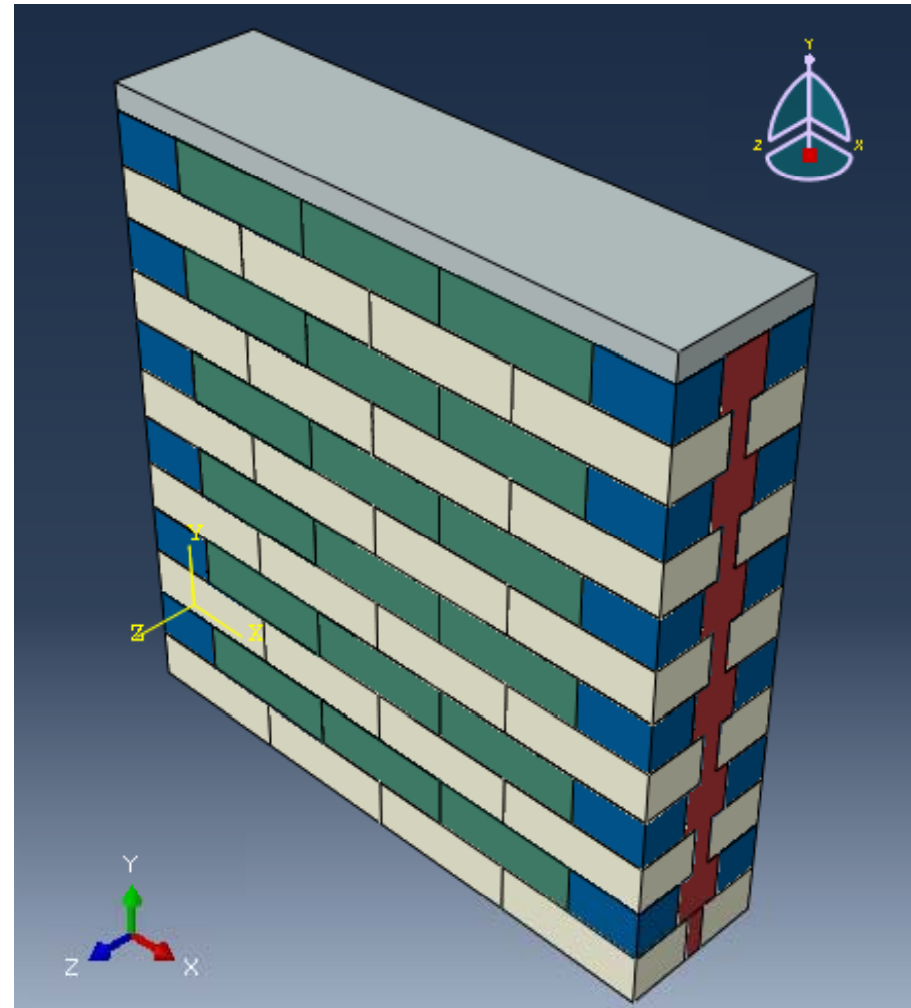


Elasto-Plastic Damage Model

Wall tested by Demir et al. (2011)



- The wall is a multilayer, leaf wall (two leaves) with rubble material as filling material between the two leaves.
- The interface between the units is dry connection.





Elasto-Plastic Damage Model

Wall tested by C. Demir et al

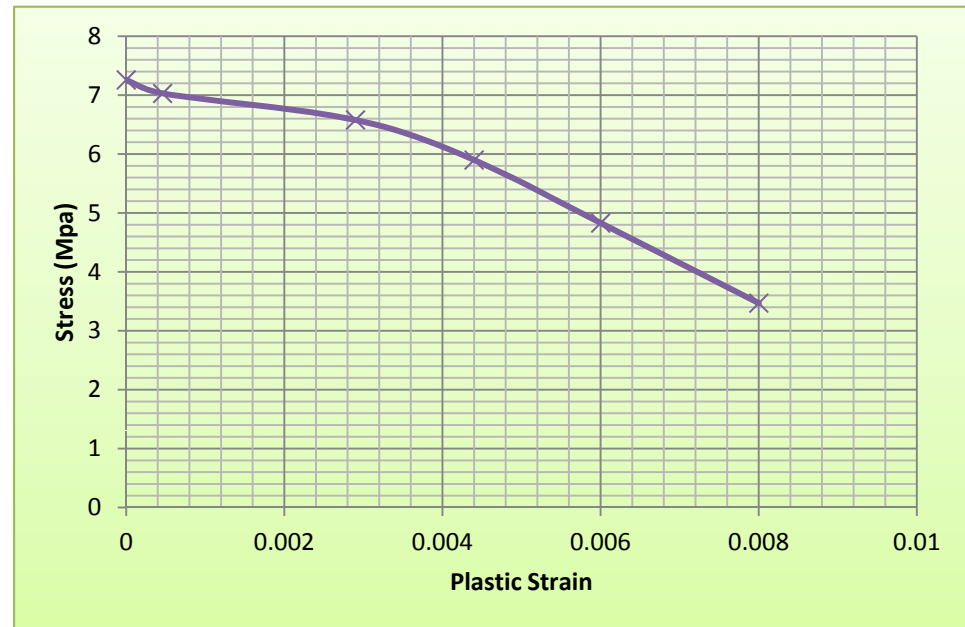
Material models used in this wall in ABAQUS

Bricks Compression:



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	fb0/fc0	K	Viscosity Parameter
2.4E-009	3200	0.18	36	0.1	1.16	0.67	0

Compression	
Plastic Strain	Stress (Mpa)
0	7.26
0.00046	7.03
0.0029	6.58
0.0044	5.9
0.006	4.83
0.008	3.47





Elasto-Plastic Damage Model

Wall tested by **C. Demir et al**

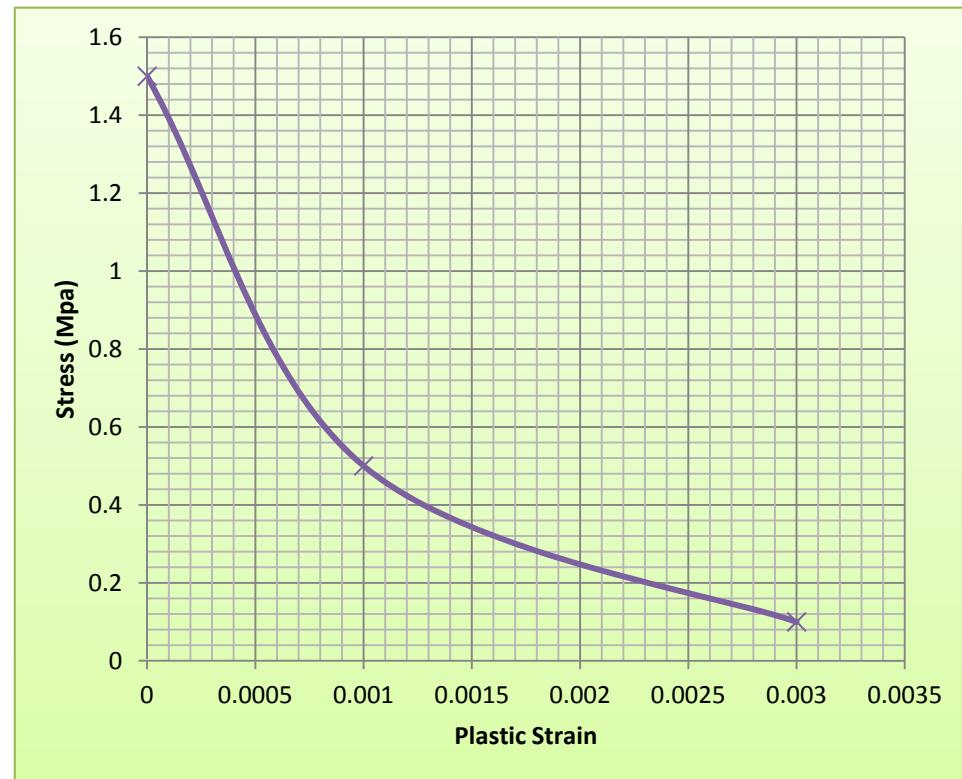
Material models used in this wall in ABAQUS

Bricks Tension:



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	fb0/fc0	K	Viscosity Parameter
2.4E-009	3200	0.18	36	0.1	1.16	0.67	0

Tension	
Plastic Strain	Stress (Mpa)
0	1.5
0.001	0.5
0.003	0.1





Elastio-Plastic Damage Model

Wall tested by **C. Demir et al**

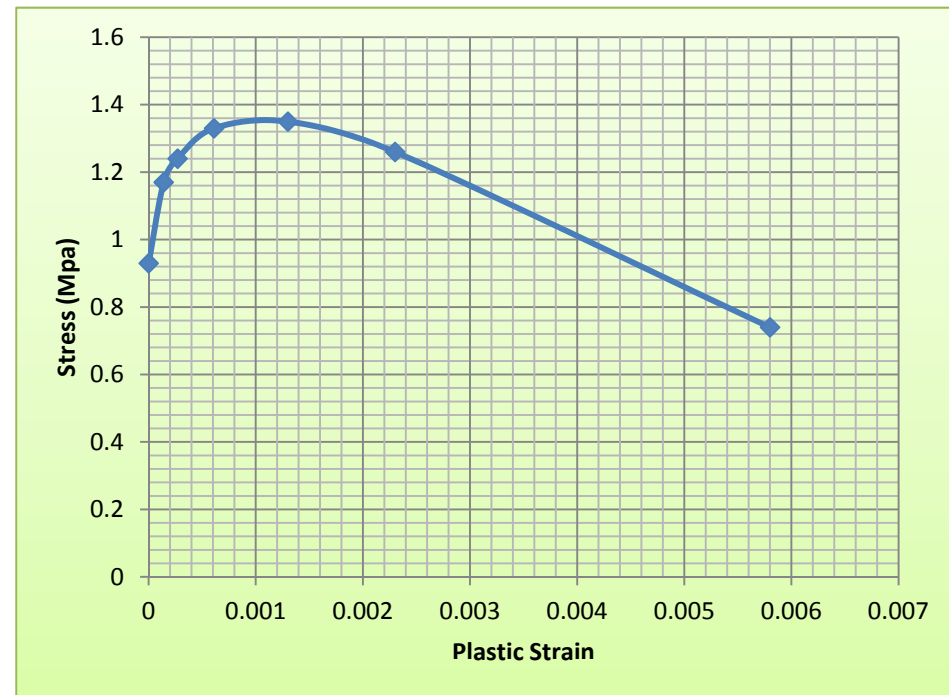
Material models used in this wall in ABAQUS

Rubble Compression:



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	fb0/fc0	K	Viscosity Parameter
1E-009	590	0.18	36	0.1	1.16	0.67	0

Compression	
Plastic Strain	Stress (Mpa)
0	0.93
0.00014	1.17
0.00027	1.24
0.00061	1.33
0.0013	1.35
0.0023	1.26
0.0058	0.74





Elasto-Plastic Damage Model

Wall tested by C. Demir et al

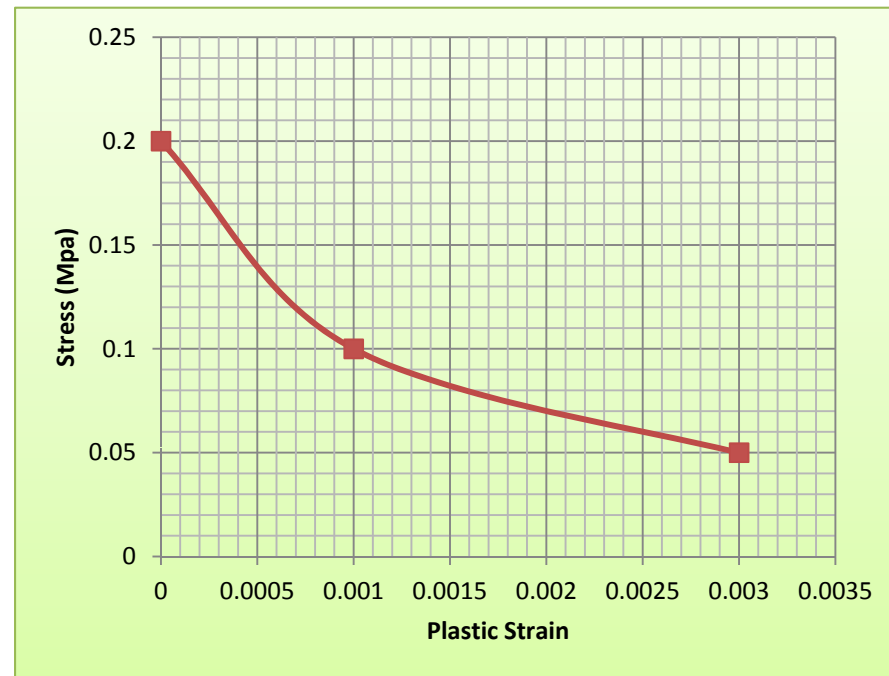
Material models used in this wall in ABAQUS

Rubble Tension:



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	f_b/f_{c0}	K	Viscosity Parameter
1E-009	590	0.18	36	0.1	1.16	0.67	0

Tention	
Plastic Strain	Stress (Mpa)
0	0.2
0.001	0.1
0.003	0.05

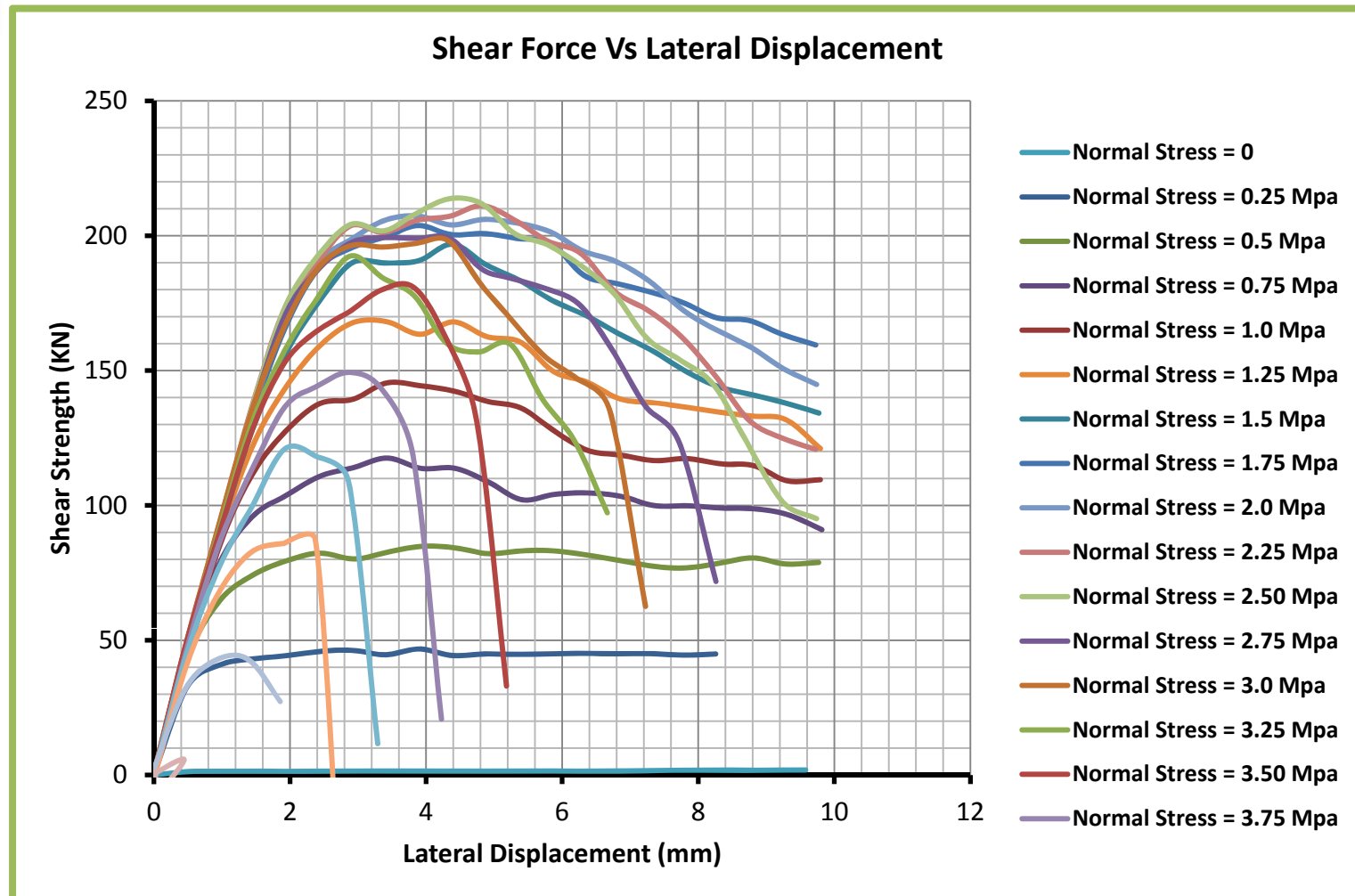




Elasto-Plastic Damage Model

Wall tested by C. Demir et al

RESULTS: Wall Lateral Strength.

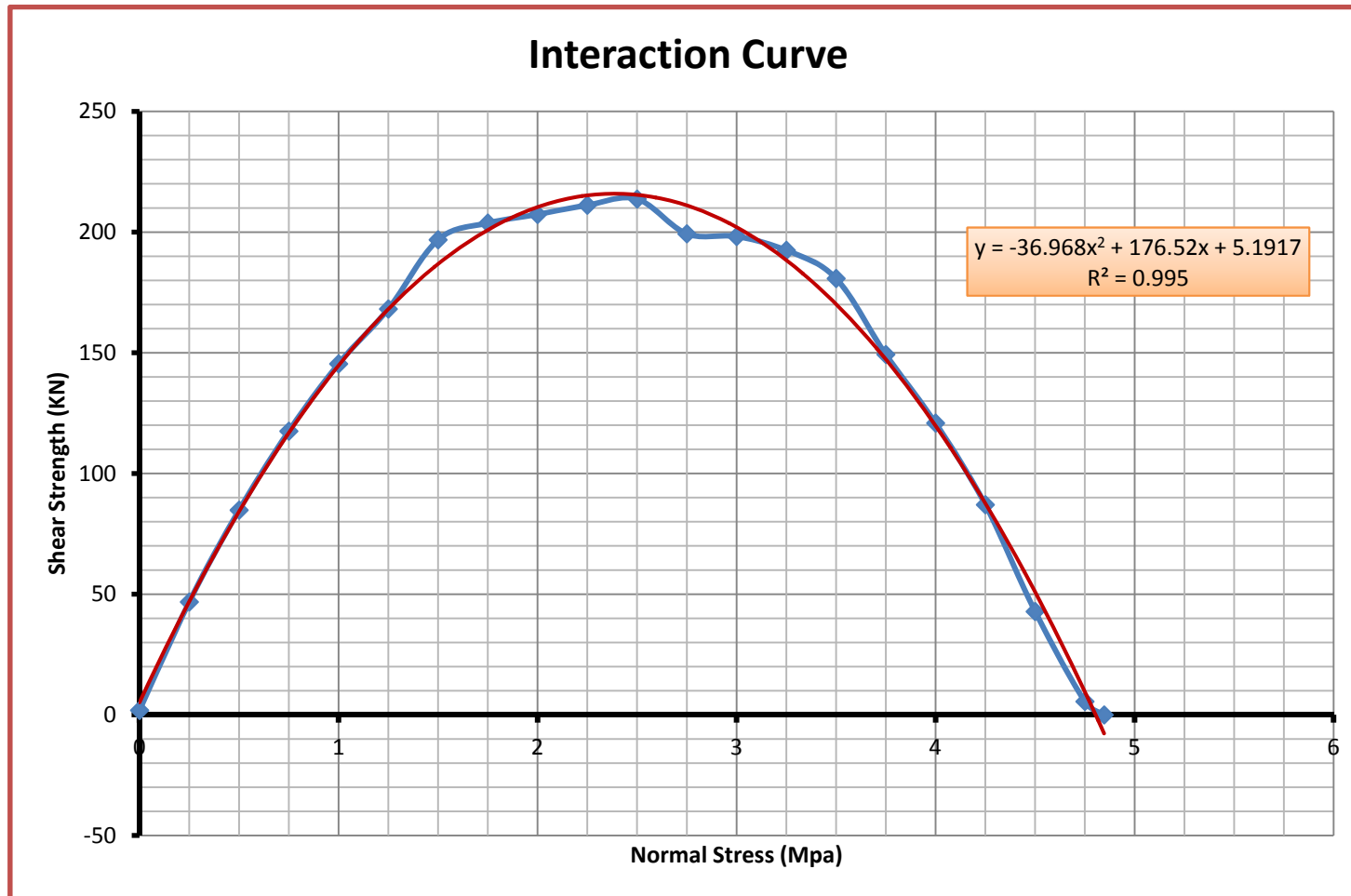




Elasto-Plastic Damage Model

Wall tested by C. Demir et al

RESULTS: Interaction Curve.

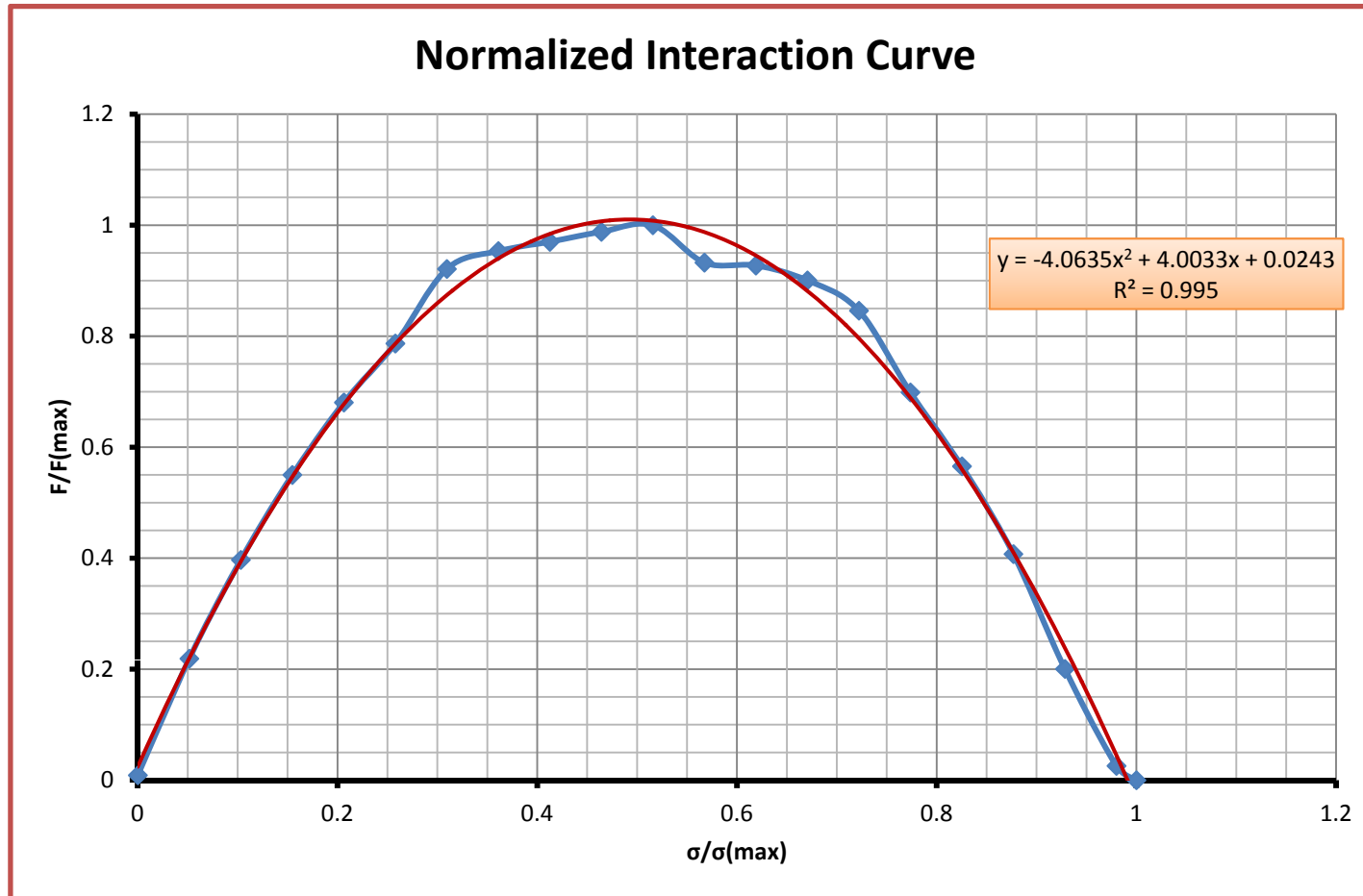




Elasto-Plastic Damage Model

Wall tested by C. Demir et al

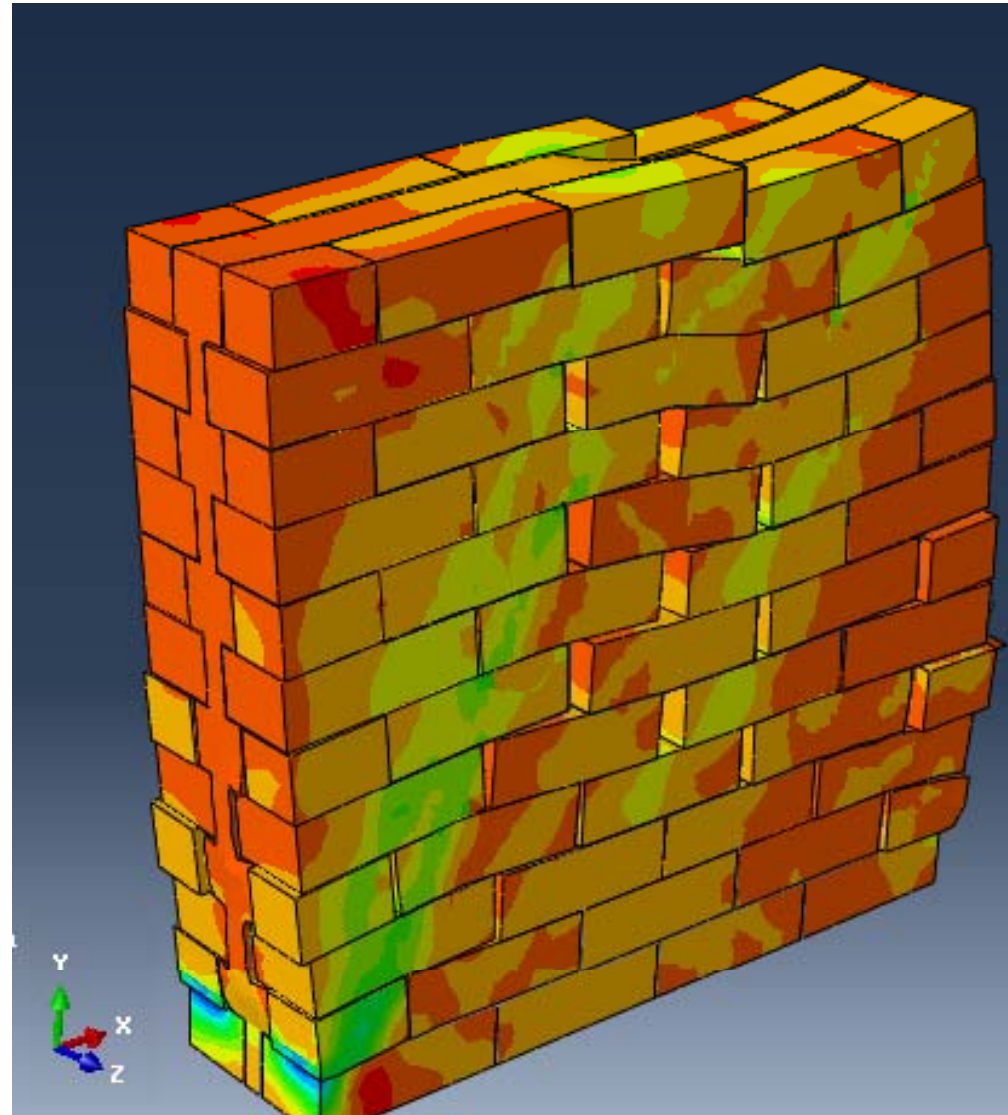
RESULTS: Normalized Interaction Curve.





FEM Simulations

Wall tested by C. Demir et al



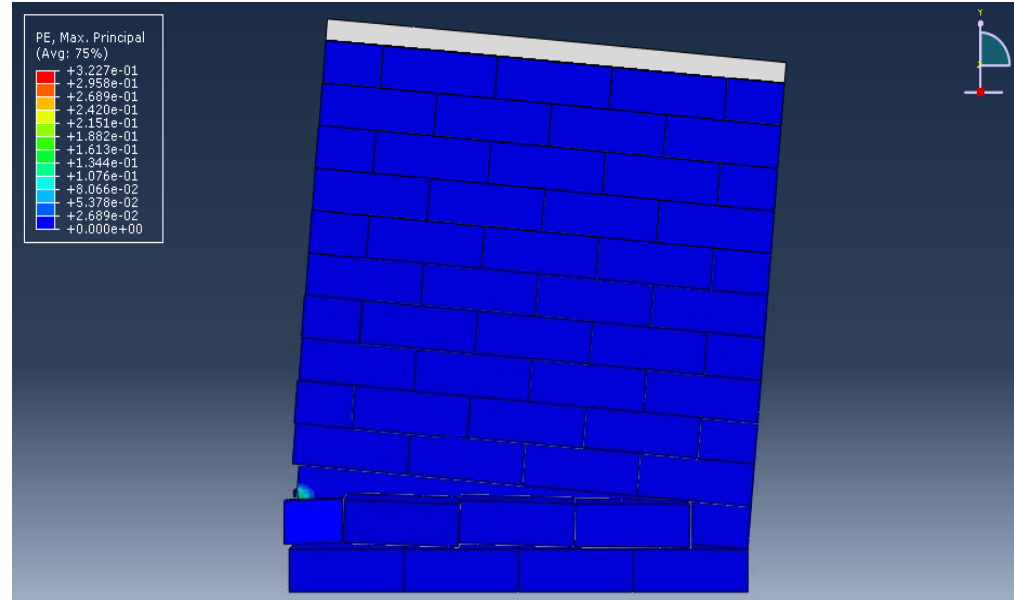


FEM Simulations

Wall tested by C. Demir et al

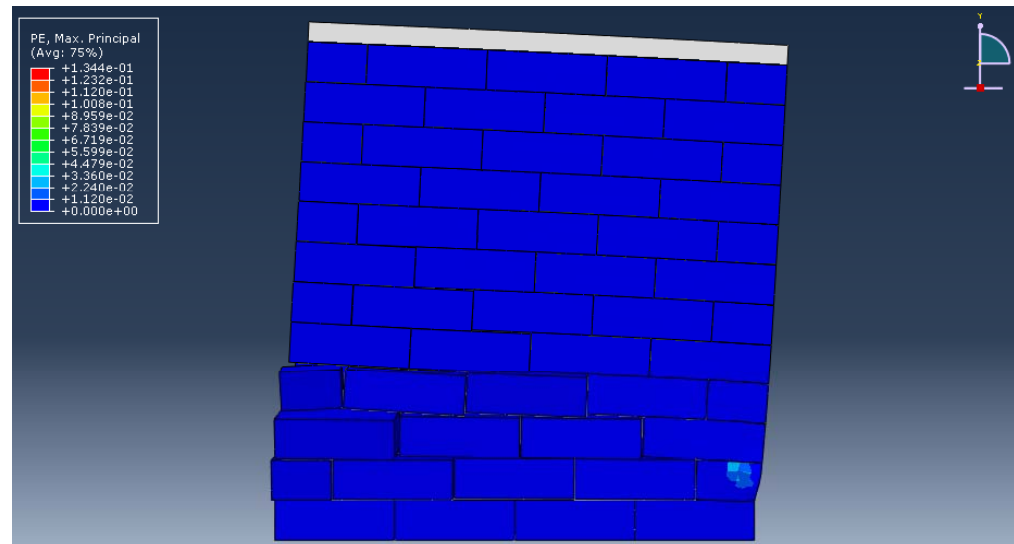


$$\sigma_n = 0 \text{ Mpa}$$



Plastic Strain

$$\sigma_n = 0.25 \text{ Mpa}$$



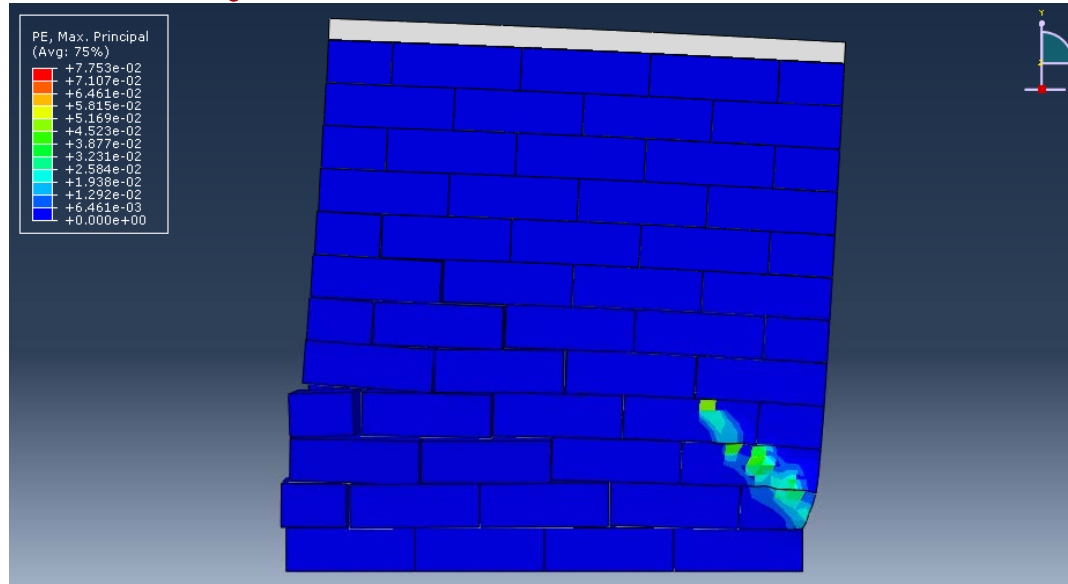


FEM Simulations

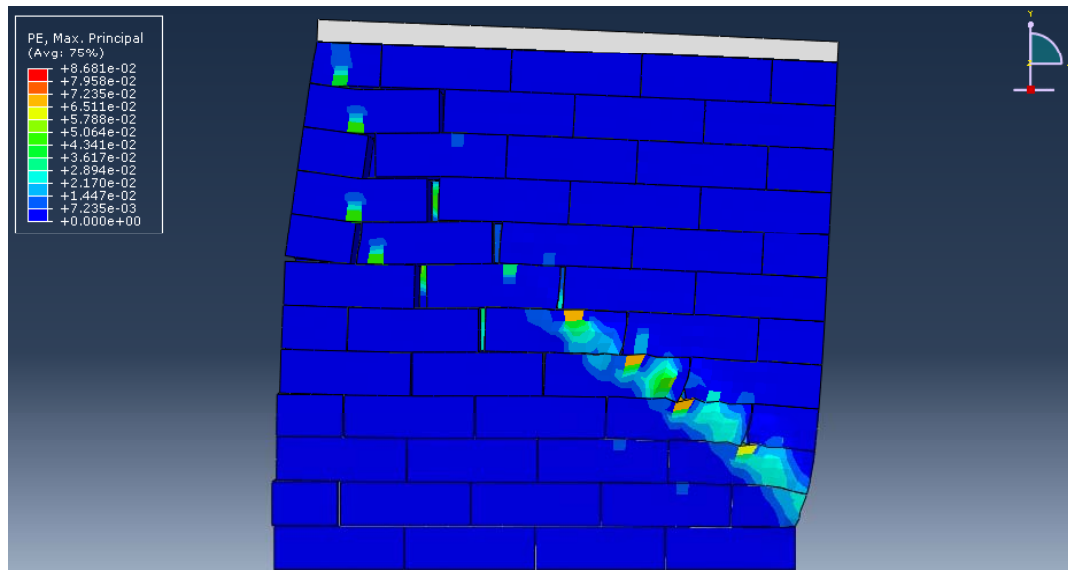
Wall tested by C. Demir et al



$$\sigma_n = 0.50 \text{ Mpa}$$



$$\sigma_n = 1.0 \text{ Mpa}$$



Plastic Strain

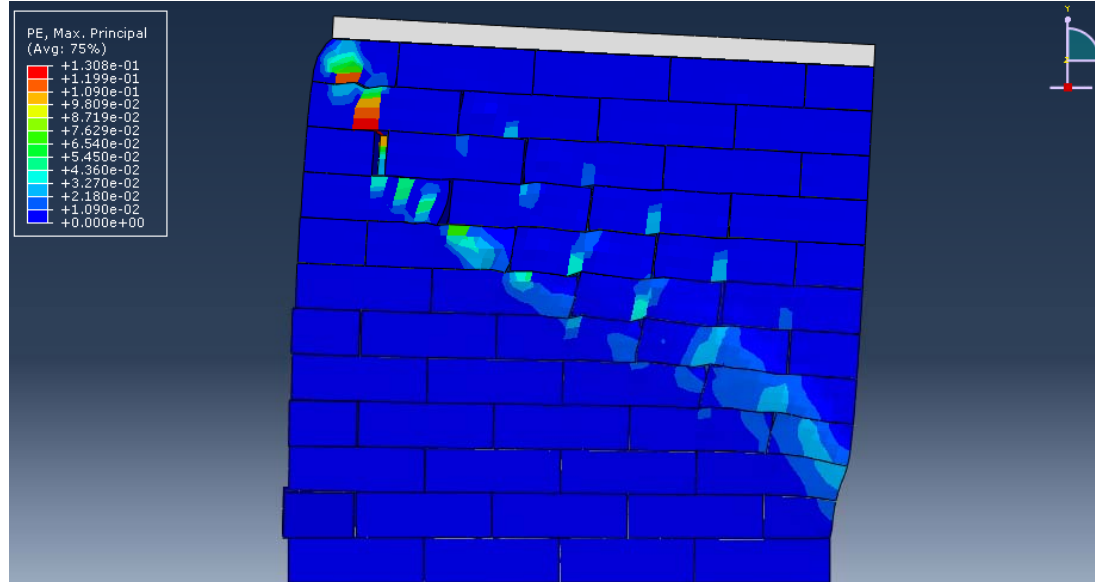


FEM Simulations

Wall tested by C. Demir et al

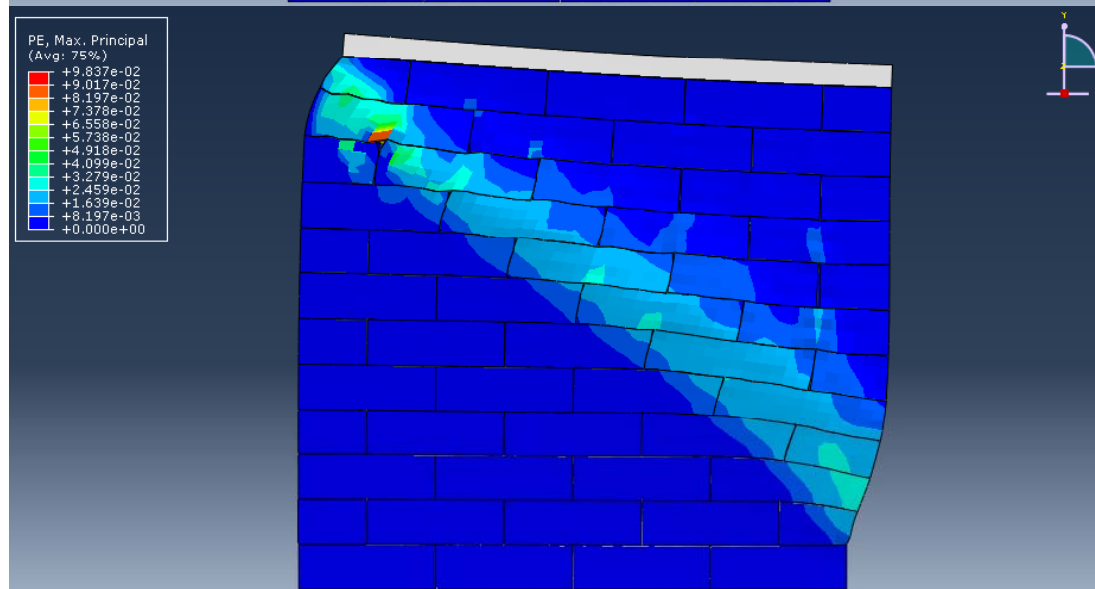


$\sigma_n = 2.0 \text{ Mpa}$



Plastic Strain

$\sigma_n = 3.0 \text{ Mpa}$



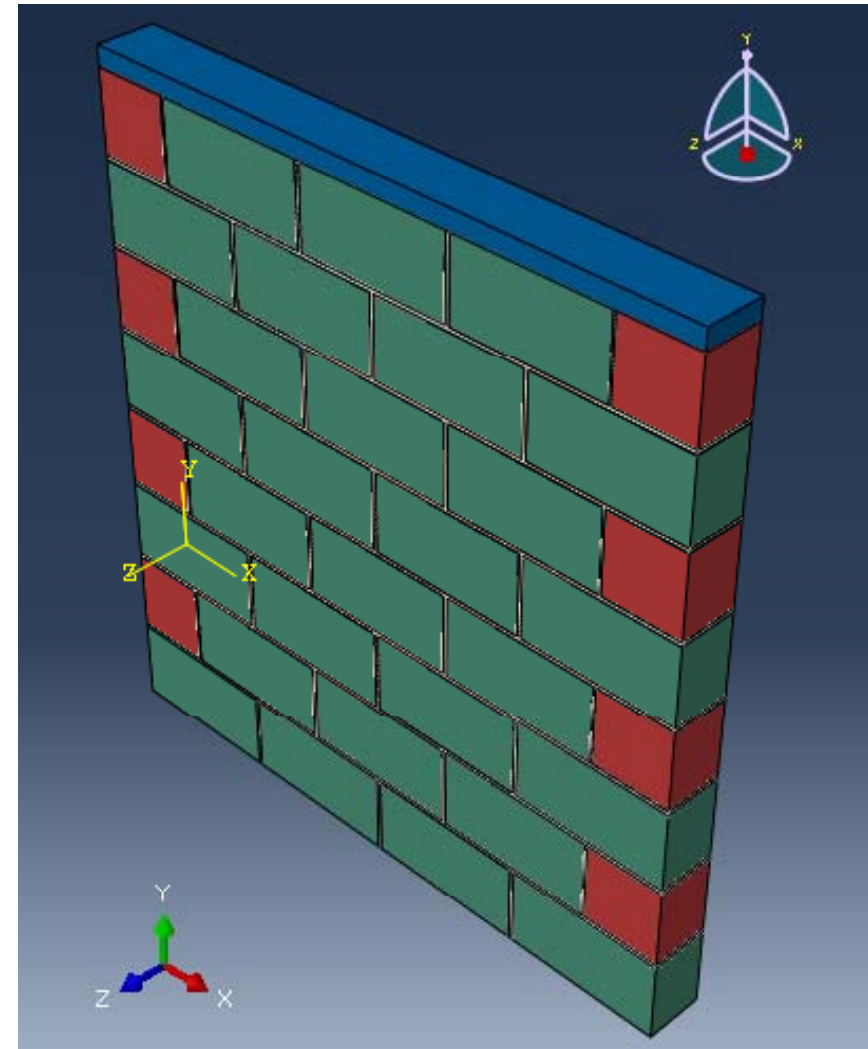


Elasto-Plastic Damage Model

Wall studied by Nanni et al. (2005)



- This wall is a single leaf wall of hollow concrete blocks.
- The interface between the bricks is mortar.





Elasto-Plastic Damage Model

Wall studied by Nanni et al. (2005)

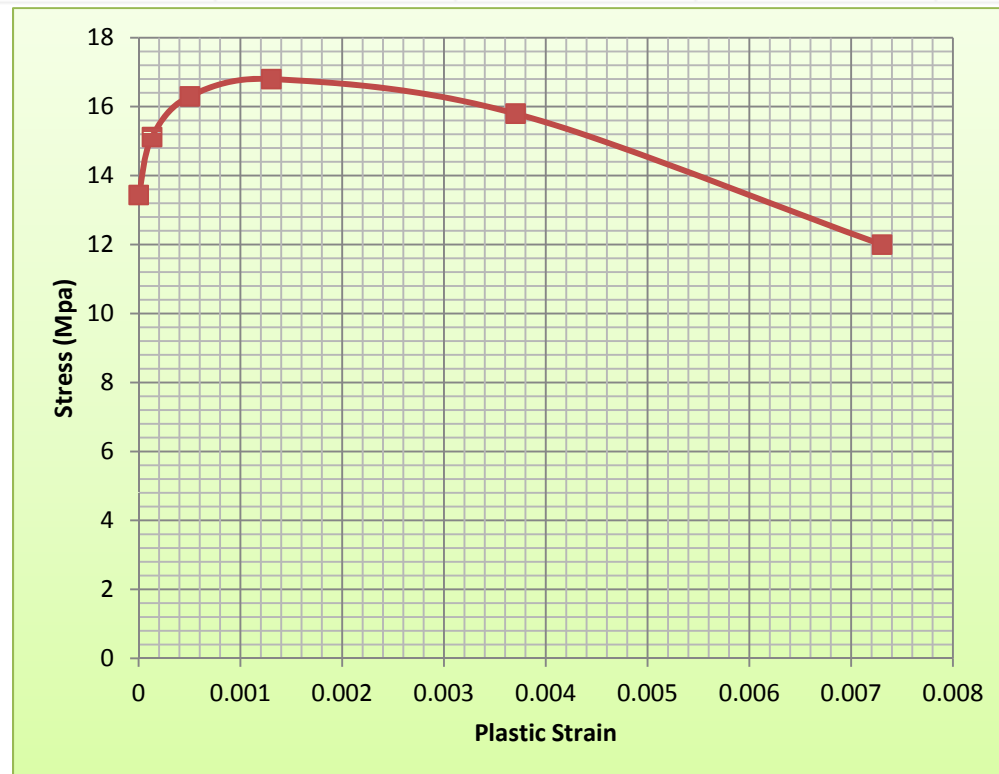
Material models used in this wall in ABAQUS

Hollow concrete blocks:



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	fb0/fc0	K	Viscosity Parameter
2.4E-009	19264.26744	0.2	36	0.1	1.16	0.67	0

Compression	
Plastic Strain	Stress (Mpa)
0	13.44
0.000131	15.12
0.000502	16.3
0.001302	16.8
0.003702	15.8
0.007302	12





Elasto-Plastic Damage Model

Wall studied by Nanni et al

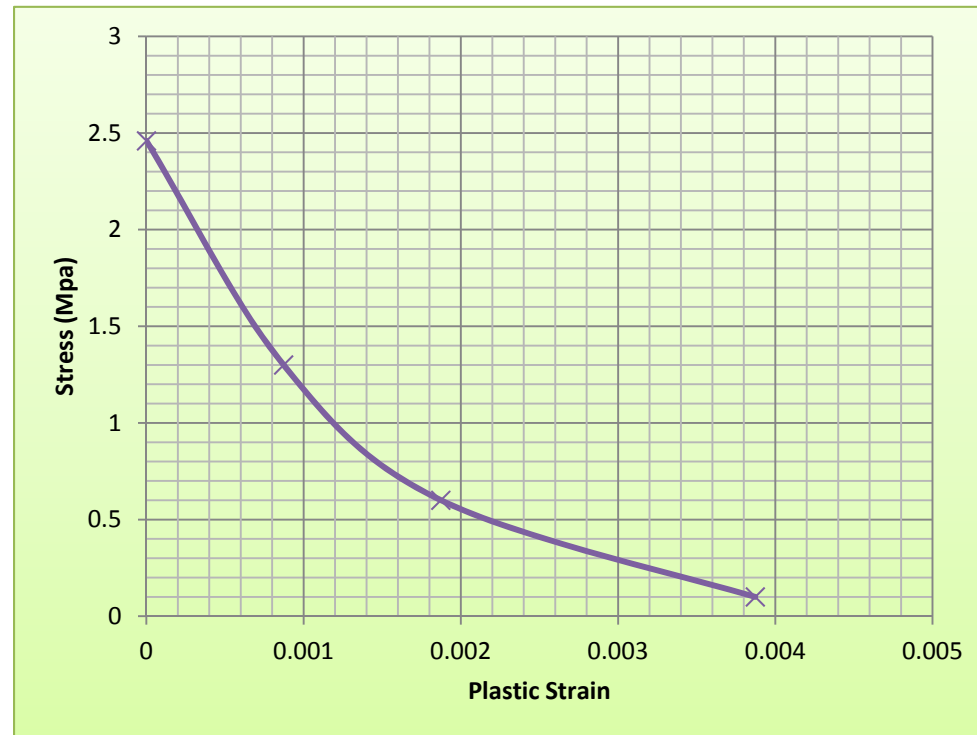
Material models used in this wall in ABAQUS

Hollow concrete blocks:



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	fb0/fc0	K	Viscosity Parameter
2.4E-009	19264.26744	0.2	36	0.1	1.16	0.67	0

Tension	
Plastic Strain	Stress (Mpa)
0	2.459268
0.000872	1.3
0.001872	0.6
0.003872	0.1





Elasto-Plastic Damage Model

Wall studied by Nanni et al

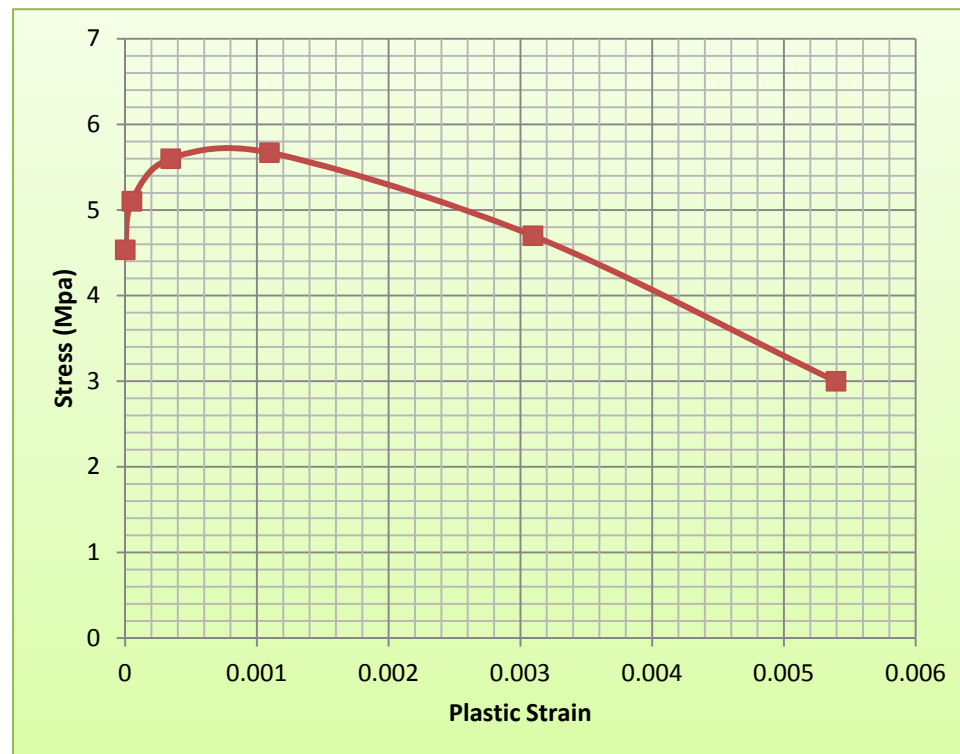
Material models used in this wall in ABAQUS

Mortar:



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	fb0/fc0	K	Viscosity Parameter
2.4E-009	11191.52805	0.2	36	0.1	1.16	0.67	0

Compression	
Plastic Strain	Stress (Mpa)
0	4.536
5.07E-05	5.103
0.000345	5.6
0.001095	5.67
0.003095	4.7
0.005395	3





Elasto-Plastic Damage Model

Wall studied by Nanni et al

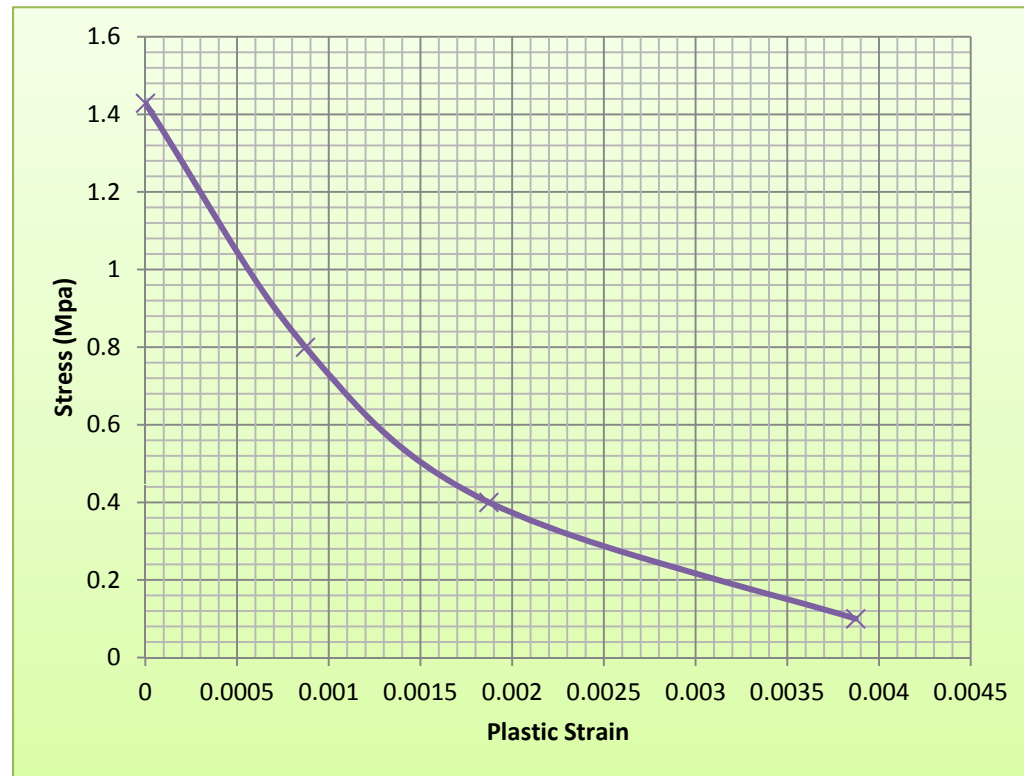
Material models used in this wall in ABAQUS

Mortar::



Mass Density	Young's Modulus	Poisson's Ratio	Dilation Angle	Eccentricity	fb0/fc0	K	Viscosity Parameter
2.4E-009	11191.52805	0.2	36	0.1	1.16	0.67	0

Tension	
Plastic Strain	Stress (Mpa)
0	1.428706
0.000872	0.8
0.001872	0.4
0.003872	0.1

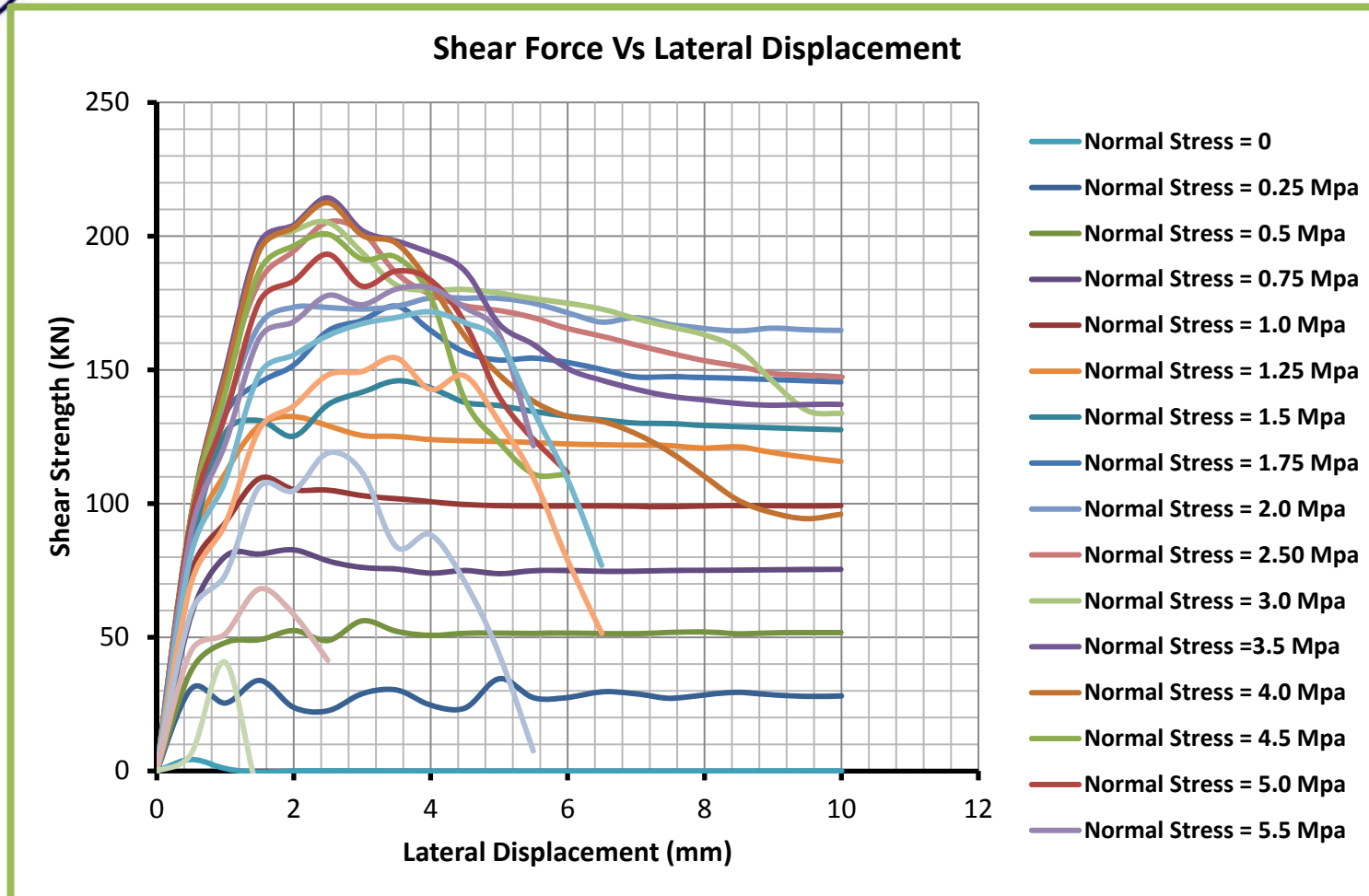




Elasto-Plastic Damage Model

Wall studied by Nanni et al

RESULTS - Wall Lateral Strength.

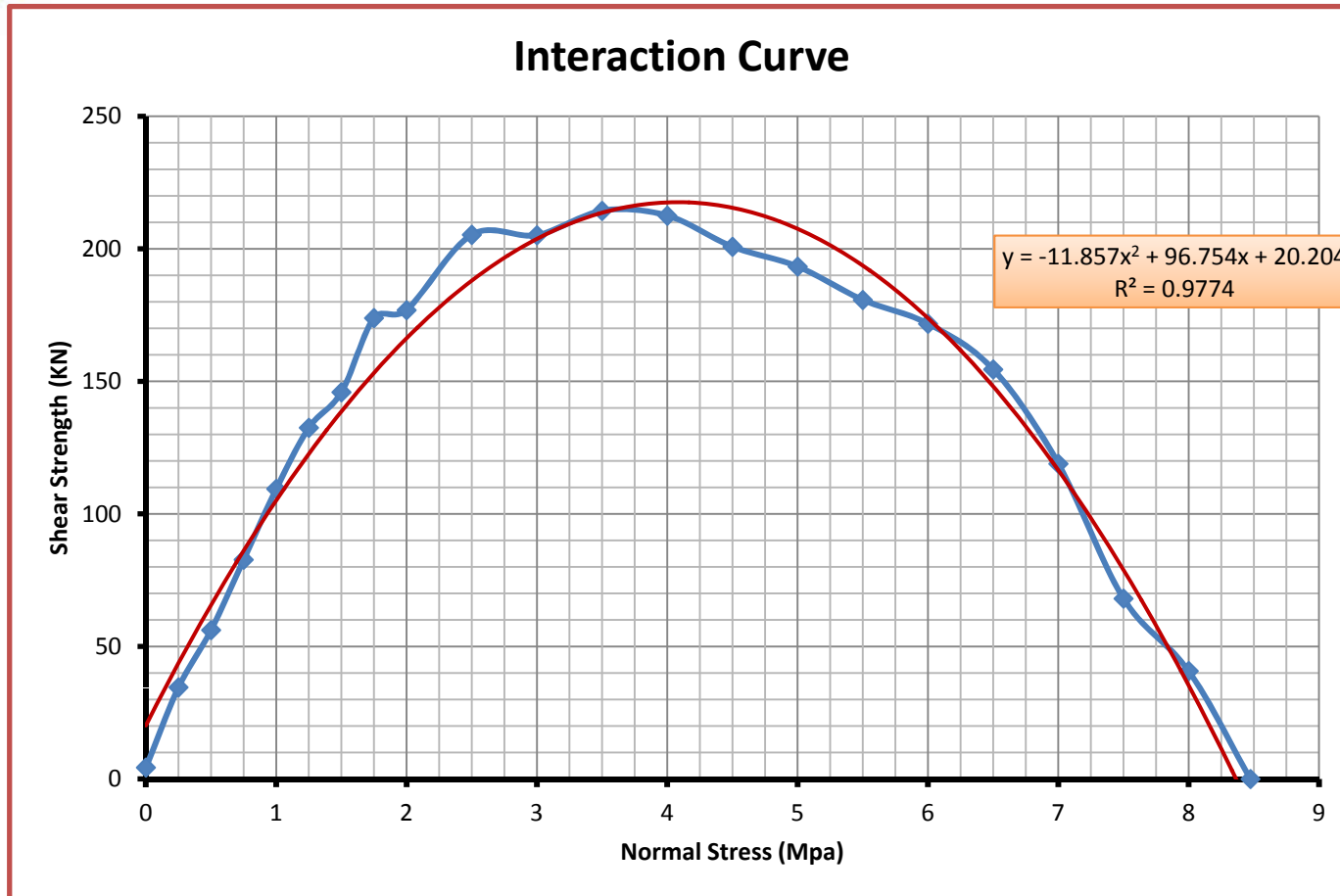




Elasto-Plastic Damage Model

Wall studied by Nanni et al

RESULTS- Interaction Curve.

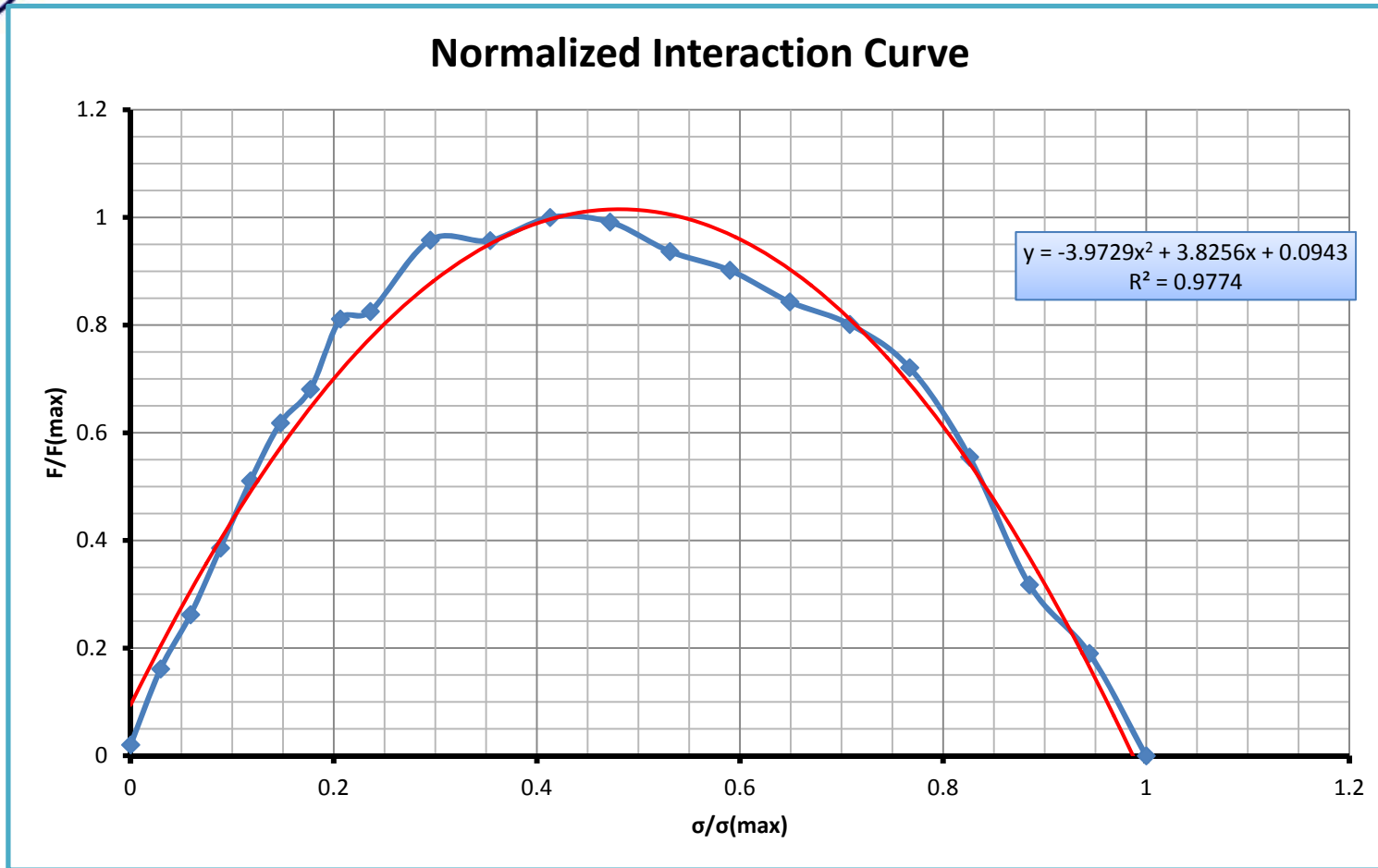




Elasto-Plastic Damage Model

Wall studied by Nanni et al

RESULTS-Normalized Interaction Curve.



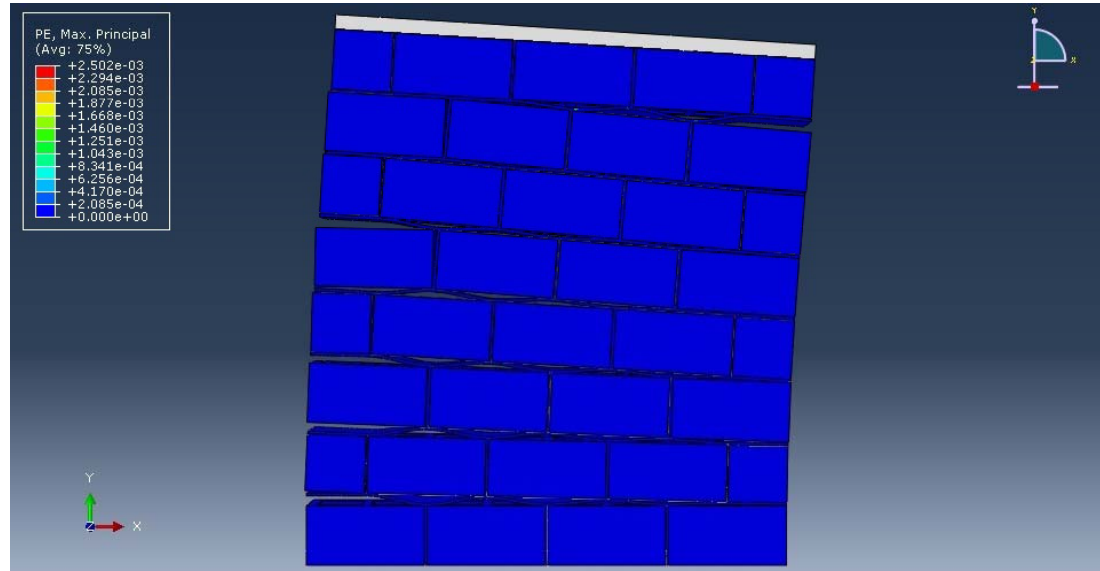


FEM Simulations

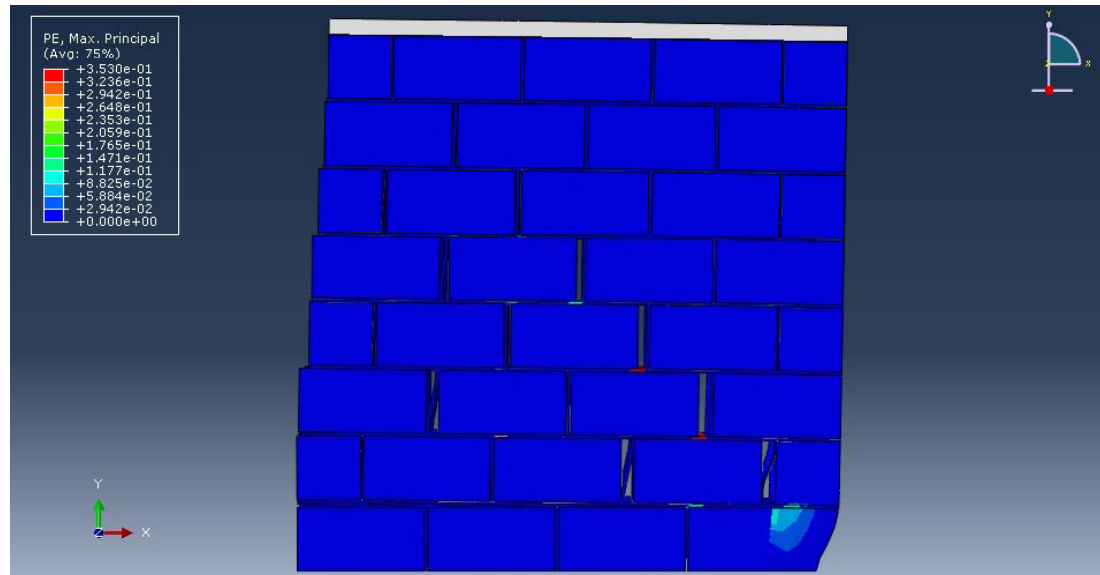
Wall studied by Nanni et al



$$\sigma_n = 0 \text{ Mpa}$$



$$\sigma_n = 0.25 \text{ Mpa}$$



Plastic Strain

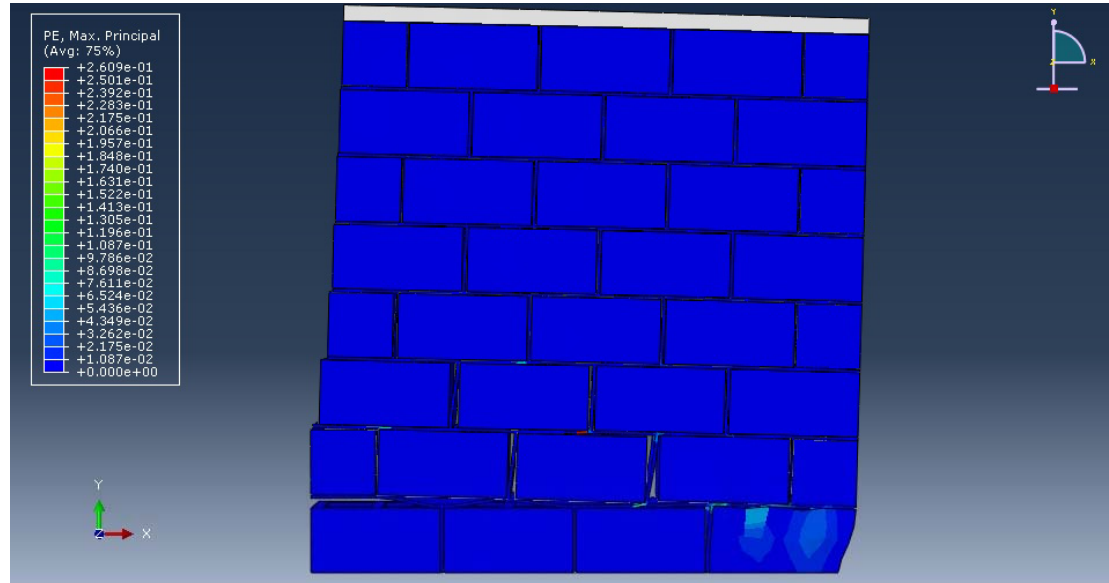


FEM Simulations

Wall studied by Nanni et al

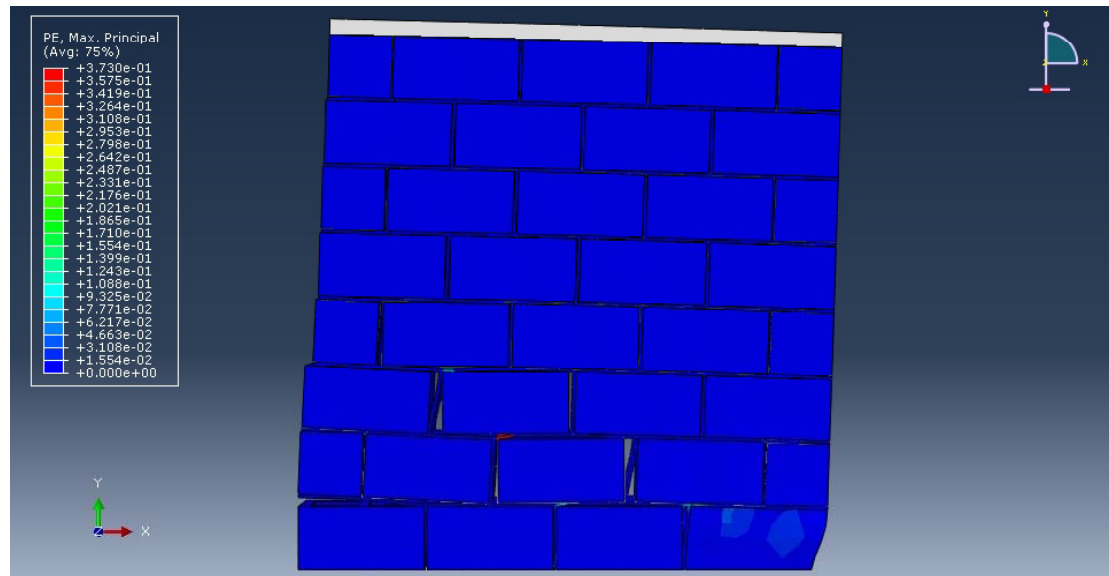


$$\sigma_n = 0.50 \text{ Mpa}$$



Plastic Strain

$$\sigma_n = 0.75 \text{ Mpa}$$



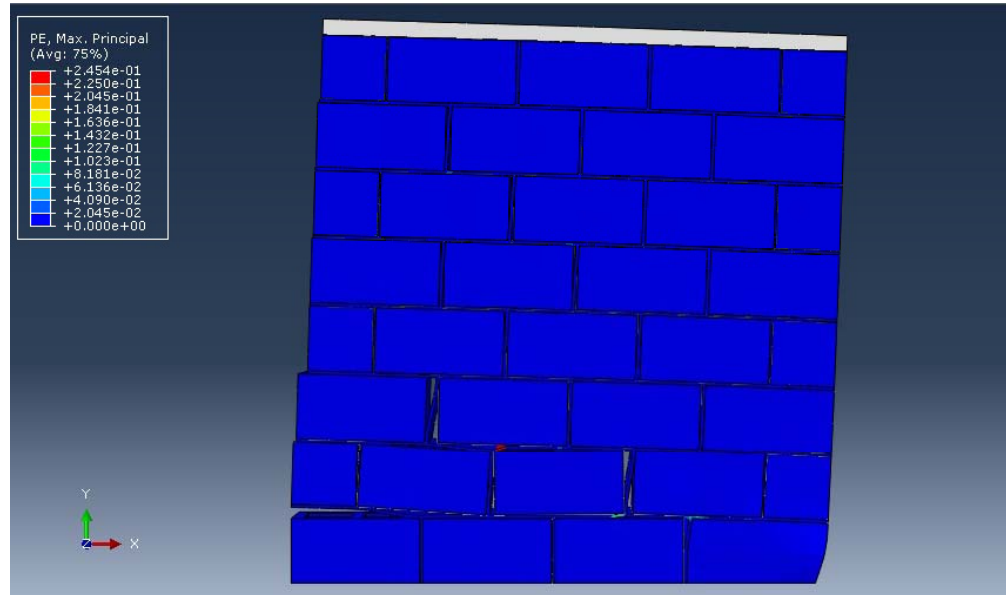


FEM Simulations

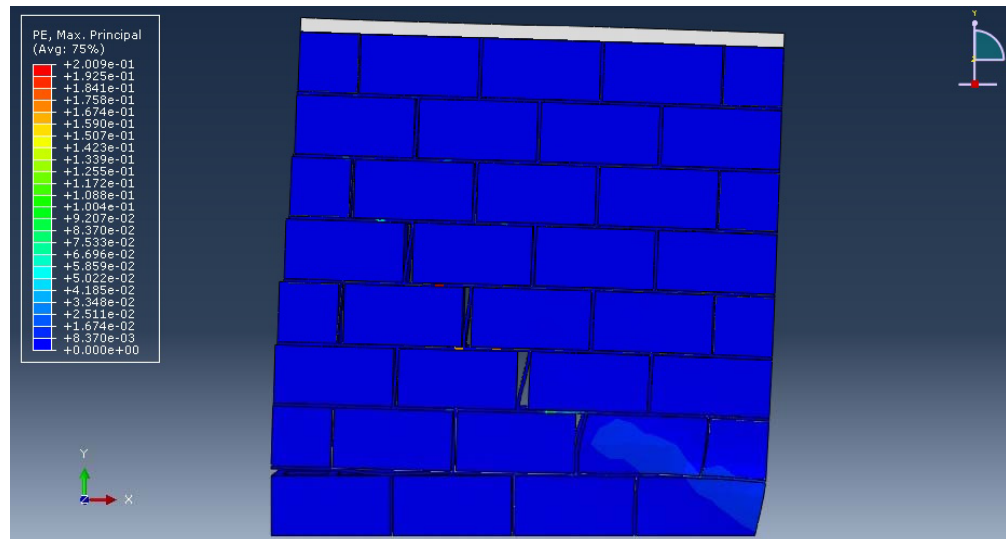
Wall studied by Nanni et al



$$\sigma_n = 1.0 \text{ Mpa}$$



$$\sigma_n = 2.0 \text{ Mpa}$$



Plastic Strain

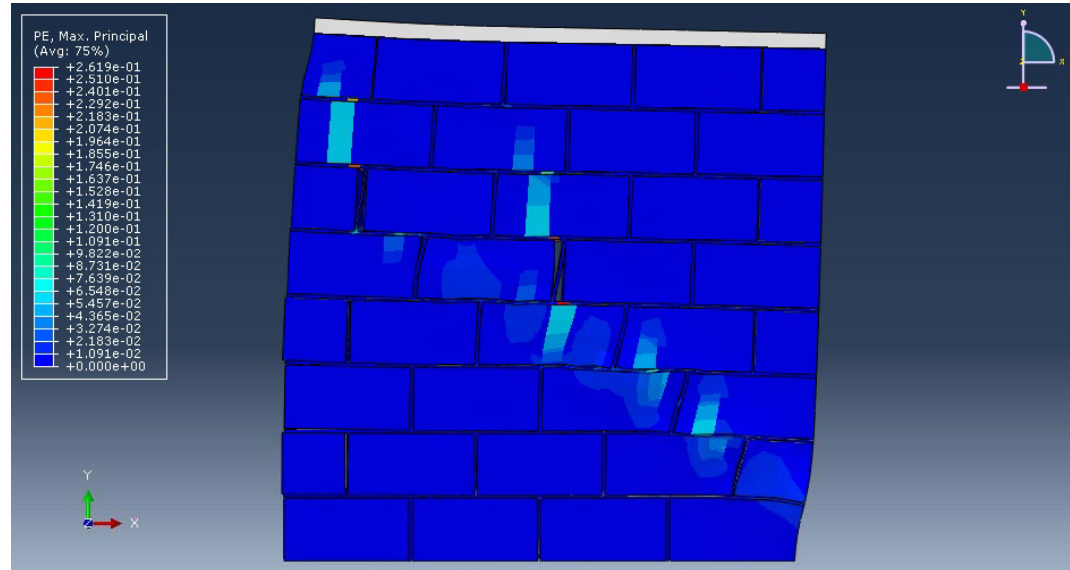


FEM Simulations

Wall studied by Nanni et al

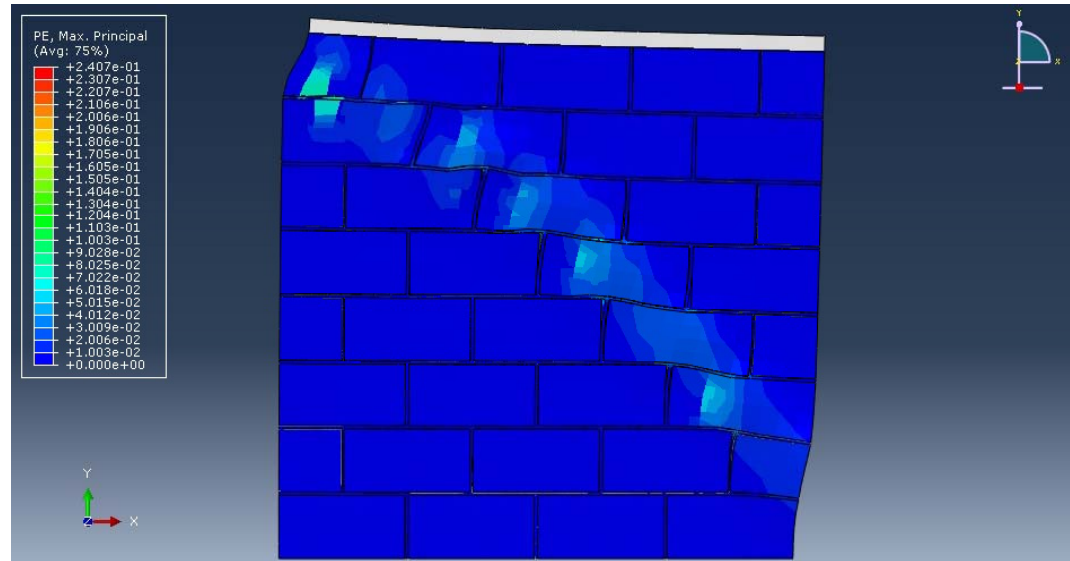


$$\sigma_n = 3.0 \text{ Mpa}$$



Plastic Strain

$$\sigma_n = 4.0 \text{ Mpa}$$



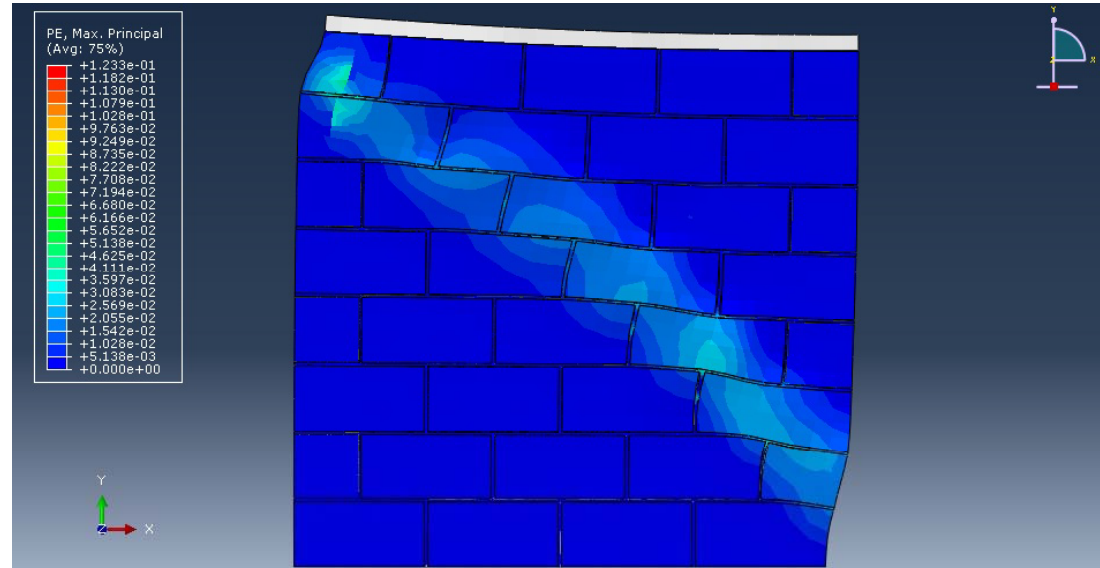


FEM Simulations

Wall studied by Nanni et al

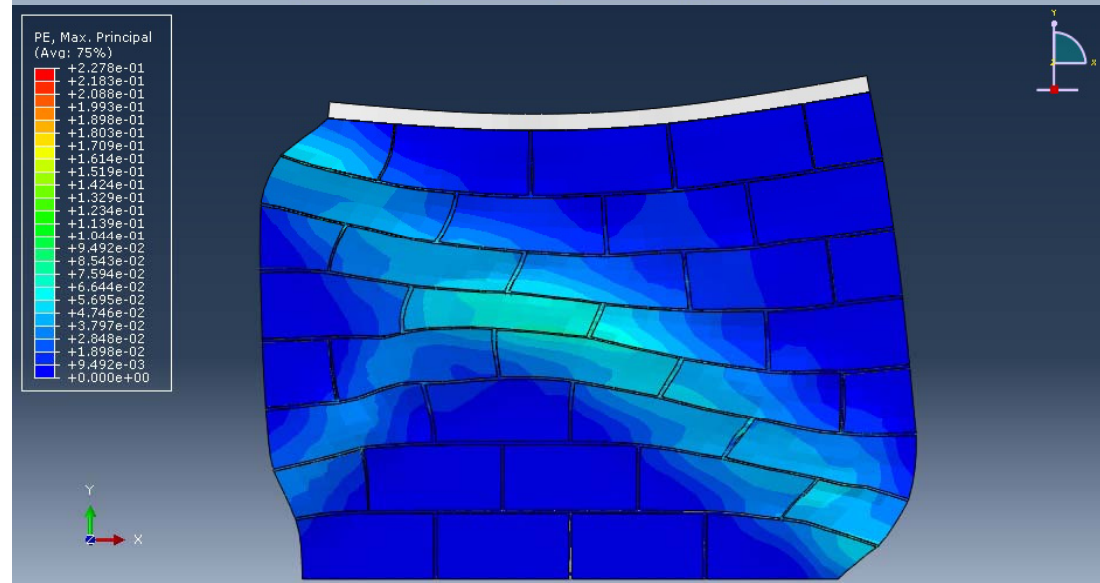


$$\sigma_n = 6.0 \text{ Mpa}$$



Plastic Strain

$$\sigma_n = 7.0 \text{ Mpa}$$

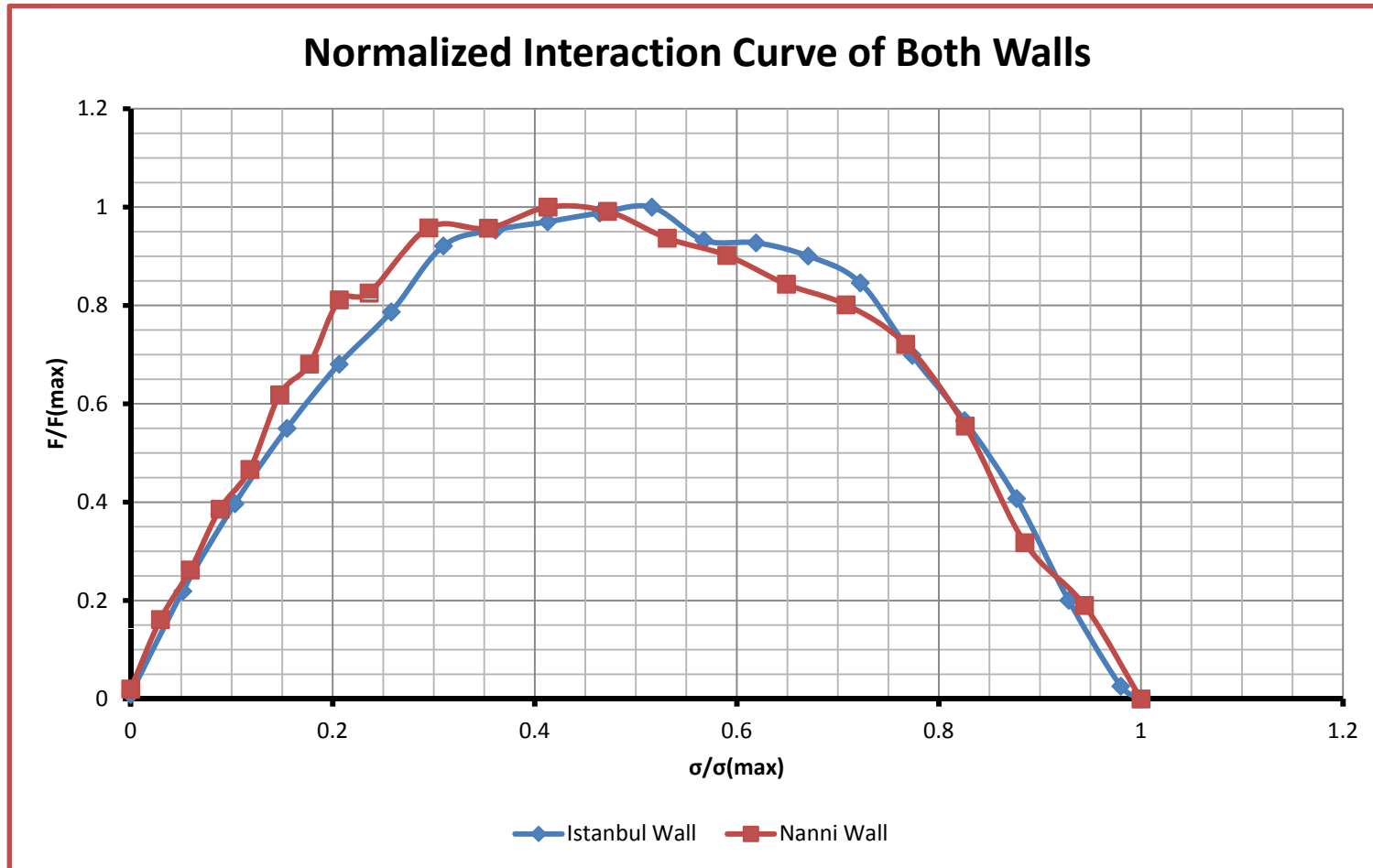




Elasto-Plastic Damage Model



Normalized Interaction Curve of both walls.

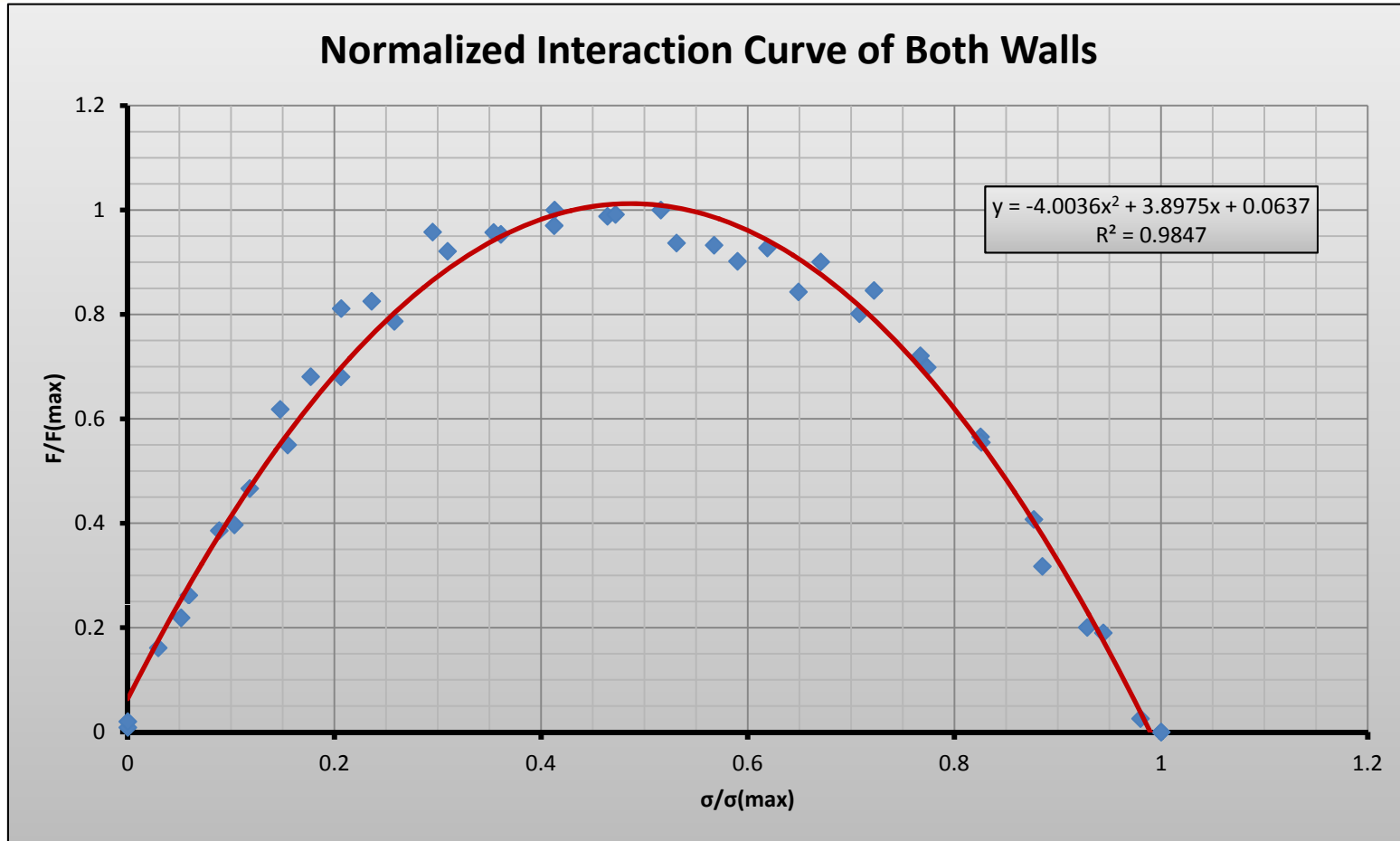




Elasto-Plastic Damage Model



Normalized Interaction Curve of both walls.





Conclusion



- Masonry structure mechanics is one of the areas where limited research has been carried out in the Gulf Region.
- There exists a strong need to initiate seismic research as seismic activities could take place at any time in the Region.
- ABAQUS shows promising features in handling analysis of complex structures and loads.



Conclusion



- The effect of axial loading on the capacity of the lateral wall is independent of the wall pattern and material used.
- Walls with aspect ratios -1.0 exhibited maximum lateral resistance when the axial loading was almost 50% of the wall axial capacity.



WORK IN PROGRESS KFUPM

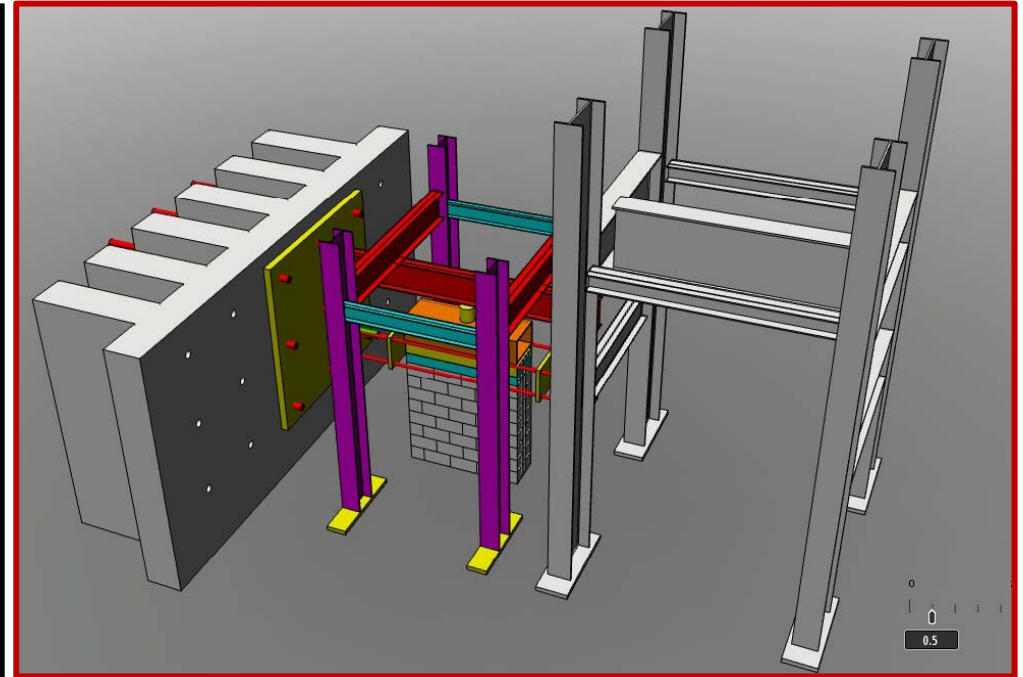


- As mentioned before, there is a cooperation between KFUPM and ITU.
- Recently, the lab facilities are being constructed in KFUPM for cyclic load testing of masonry walls.
- In Numerical Modeling work on Nonlinear FEA of CFRP Retrofitted masonry walls is in progress



WORKS IN KFUPM

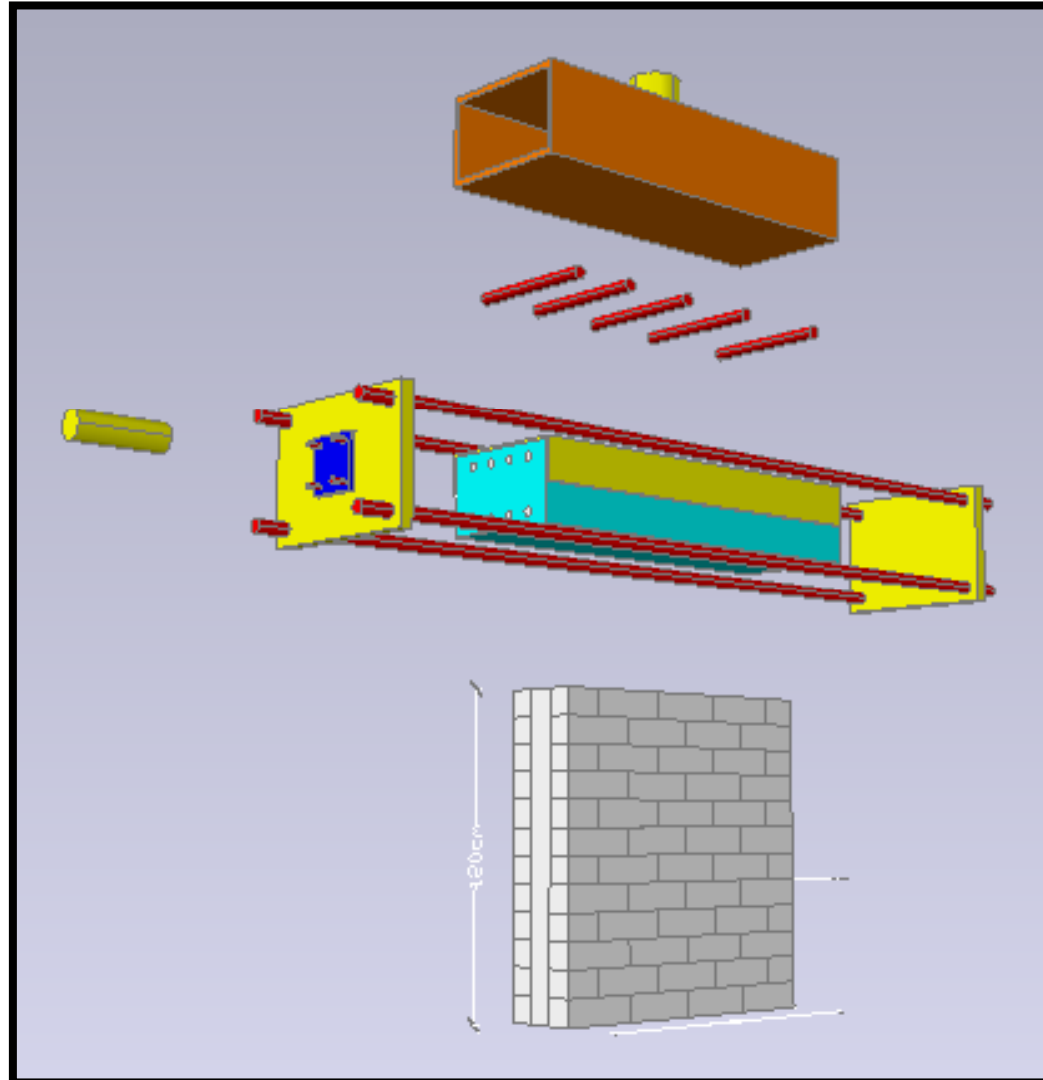
Reaction wall and loading frame.





WORKS IN KFUPM

Wall testing set up





Acknowledgments



- Experimental study was funded by ITU Research Fund.
- The study being carried out in KFPUM is funded by King Fahd University of Petroleum & Minerals under project number IN101016 “ Seismic Retrofit of Typical Wall Systems in Traditional Structures in the Kingdom” which is gratefully acknowledged.
- The authors acknowledge the mentoring provided by ITU to the KFUPM graduate students involved in this project



Thank you for your attention